



## **Thurrock Flexible Generation Plant**

**Preliminary Environmental Information Report  
Appendix 15.1: Flood Risk Assessment**

**Date:** September 2018

**Environmental Impact Assessment**  
**Preliminary Environmental Information Report**

**Volume 6**  
**Appendix 15.1**

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## Summary

This document provides a development-specific Flood Risk Assessment (FRA) in accordance with the requirements of the National Planning Policy Framework (NPPF) and Planning Practice Guidance (PPG ID7) for the proposed Thurrock Flexible Generation Plant.

## Qualifications

This document has been prepared by Jonathan Morley, a Principal Consultant, who has 12 years' experience of environmental impact assessment.

It has been checked by Richard Chalmers who is a Chartered Engineer and Royal Academy of Engineering Teaching Fellow, with over 30 years' professional experience in consultancy in the UK.

# 1. Introduction

## 1.1 Background

- 1.1.1 A site-specific Flood Risk Assessment (FRA) has been prepared for the Thurrock Flexible Generation Plant, specifically the main development site which is referred to as Zone A and encompasses the proposed gas engine, battery storage and substation facilities.
- 1.1.2 A review of the flood risk posed to the above-ground installation for gas connection to Feeder 18 (within Zone E) has also been undertaken to aid in informing flood resilience requirements. The size and scale of development in Zone E would have negligible impact on the local hydrological regimes.
- 1.1.3 The FRA has been produced in accordance with the Overarching National Policy Statement (NPS) for Energy EN-1, the National Planning Policy Framework (NPPF) and Planning Practice Guidance (PPG) ID7 and relevant local planning policies, a summary of each is presented in Section 3. The policies cover the requirements in respect to Nationally Significant Infrastructure Projects.
- 1.1.4 The FRA supports the Development Consent Order (DCO) application for Thurrock Flexible Generation Plant in accordance with the Infrastructure Planning (Applications: Prescribed Forms and Procedure) Regulations 2009 (as amended). It also forms an appendix to Thurrock Flexible Generation Plant Environmental Statement Volume 3, Chapter 15: Hydrology and Flood Risk.
- 1.1.5 Developments that are designed without regard to flood risk may endanger lives, damage property, cause disruption to the wider community, damage the environment, be difficult to insure and require additional expense on remedial works.
- 1.1.6 Current guidance on development and flood risk (PPG: ID7 Flood risk and coastal change) identifies several key aims for a development to ensure that it is sustainable in flood risk terms. These aims are as follows:
- the development should not be at a significant risk of flooding and should not be susceptible to damage due to flooding;
  - the development should not be exposed to flood risk such that the health, safety and welfare of the users of the development, or the population elsewhere, is threatened;
  - normal operation of the development should not be susceptible to disruption as a result of flooding;

- safe access to and from the development should be possible during flood events;
- the development should not increase flood risk elsewhere;
- the development should not prevent safe maintenance of watercourses or maintenance and operation of flood defences;
- the development should not be associated with an onerous or difficult operation and maintenance regime to manage flood risk. The responsibility for any operation and maintenance required should be clearly defined;
- future users of the development should be made aware of any flood risk issues relating to the development;
- the development design should be such that future users will not have difficulty obtaining insurance or mortgage finance, or in selling all or part of the development, as a result of flood risk issues;
- the development should not lead to degradation of the environment; and
- the development should meet all of the above criteria for its entire lifetime, including consideration of the potential effects of climate change.

1.1.7 The FRA is undertaken with due consideration of these sustainability aims.

1.1.8 The key objectives of the FRA are:

- to assess the flood risk to the proposed development and to demonstrate the feasibility of appropriately designing the development such that any residual flood risk to the development and users would be acceptable;
- to assess the potential impact of the proposed development on flood risk elsewhere and to demonstrate the feasibility of appropriately designing the development, such that the development would not increase flood risk elsewhere; and
- to satisfy the requirements of the NPS, the NPPF and PPG and DCO application guidance insofar as they require FRAs to be submitted in support of DCO applications.

## 1.2 Methodology

- 1.2.1 The proposed study area of the FRA follows the Thurrock Flexible Generation Plant hydrology and flood risk study area as defined in Volume 3, Chapter 15: Hydrology and Flood Risk. It includes a 1 km buffer around Zone A.
- 1.2.2 The buffers applied are considered appropriate for data collection taking into account the nature of proposed development and likely zone of influence on hydrological receptors.

1.2.3 In order to achieve the objectives outlined within 1.1.8, a staged approach was adopted in undertaking the FRA in accordance with NPS (EN-1), the NPPF and PPG. Initially, screening studies have been undertaken utilising publicly available information, records and data to identify whether there are any potential sources of flooding within Zone A and elsewhere in the proposed development hydrology and flood risk study area, which may warrant further consideration. Identified potential flooding issues are then assessed further within a specific flood risk section. The aims of the assessment are:

- to review all available information and provide a qualitative analysis of the flood risk to the proposed development; and
- to identify any impact of the proposed development on flood risk elsewhere.

### 1.3 Report structure

1.3.1 This report has the following structure:

- Section 2 identifies the sources of information that have been consulted in preparation of the FRA;
- Section 3 sets out relevant legislation, guidance and local planning policy; and
- Section 4 provides the development specific FRA for the proposed permanent surface structures.

1.3.2 A hydrological review of the proposed development; requirements of the NPPF and PPG; a description of the flood risk management measures incorporated into the design of the Thurrock Flexible Generation Plant are presented below.

### 1.4 Information Sources

1.4.1 The information used in the preparation of report is set out in Table 1.1.

**Table 1.1: Information sources consulted during the preparation of the report.**

Source	Data	Information consulted/provided
Site setting and hydrology	Ordnance Survey (OS) 1:50,000 Landranger 177: East London, Billericay & Gravesend	Area information, rivers and other watercourses, general site environs, built environment
Geology, hydrogeology	British Geological Survey (BGS) 1: 50 000 digital mapping	Site and area geology, groundwater vulnerability.
Environment Agency (EA)	EA – on line mapping.	Current flood risk, local flood defences, flood levels, supplementary geology and groundwater information.

Local Planning Authority (LPA)	Thurrock Borough Council	Strategic Flood Risk Assessment, April 2009, Flood Zoning.
Sewerage/Water	Essex & Suffolk Water.	Private system, no public asset data.
Flood Risk Assessment and Planning Guidance	NPPF (2018) Planning Practice Guidance ID7 EA Fluvial Design Guide, 2009.	Provides best practice guidelines for the generation of hydraulic
Previous consultancy reports	Thurrock Strategic Flood Risk Assessment, Level 1 Report, Final Report, September 2009.	Lead Local Flood Authority Flood Risk Assessment and mapping.
	South Essex Catchment Flood Management Plan Summary Report December 2009.	The Catchment Flood Management Plan (CFMP) provides an overview of the flood risk in the South Essex area and sets out our preferred plan for sustainable flood risk management over the next 50 to 100 years.
	Thurrock Strategic Flood Risk Assessment, Level 2 Report, Final Report, February 2010.	Lead Local Flood Authority detailed Flood Risk Assessment and mapping to inform Local Plan site allocation.

## 2. Legislation and Guidance

### 2.1 National Policies and Guidance

#### National Planning Policy Framework (July 2018 Update).

- 2.1.1 The NPPF sets out Government planning policies for England and how these are expected to be applied. The framework acts as guidance for local planning authorities and decision-takers, both in drawing up plans and making decisions about planning applications.
- 2.1.2 Paragraphs 163 -165 set out the need for an appropriate assessment of flood risk on a site specific basis. Guidance on the minimum requirements for such an assessment is contained in PPG ID7.
- 2.1.3 The NPPF requires the application of a sequential, risk-based approach to determining the suitability of land for development in flood risk areas, and that flood risk assessment should be carried out to the appropriate degree, at all levels of the planning process.

#### Planning Practice Guidance, online.

- 2.1.4 PPG ID7 Flood Risk and Coastal Change provides guidance to ensure the effective implementation of the NPPF planning policy for development in areas at risk of flooding.
- 2.1.5 PPG ID7 states that a site-specific FRA is required for all proposals for new development in Flood Zones 2 and 3 and for any proposal of 1 hectare or greater in Flood Zone 1. An FRA should consider vulnerability to flooding from other sources as well as from river and sea flooding, and also the potential for any increased risk of flooding elsewhere resulting from a development.

#### Climate Change

- 2.1.6 The NPPF sets out how the planning system should help minimise vulnerability and provide resilience to the impacts of climate change. NPPF and supporting planning practice guidance on Flood Risk and Coastal Change explain when and how flood risk assessments should be used. This includes demonstrating how flood risk will be managed now and over the development's lifetime, taking climate change into account.

- 2.1.7 In February 2016 the Environment Agency (EA) updated advice on climate change allowances to support NPPF. New guidance requires that flood risk assessments and strategic flood risk assessments, assess both the central and upper end allowances (Table 2.1) to understand the range of impact.

Table 2.1: Change to extreme rainfall intensity compared to a 1961-90 baseline

Applies across all of England	Total potential change anticipated for '2020s' (2015- 39)	Total potential change anticipated for '2050s' (2040- 2069)	Total potential change anticipated for the '2080s' (2070-2115)
Upper Estimate	10%	20%	40%
Central Estimate	5%	10%	20%

#### Sea level allowances

- 2.1.8 The guidance provides a single regional allowance for each epoch or time frame for sea level rise, summarised in Table 2.2.

Table 2.2: Sea level allowance for each epoch in millimetres (mm) per year with cumulative sea level rise for each epoch in brackets (use 1990 baseline)

Area of England	1990 to 2025	2026 to 2055	2056 to 2085	2086 to 2115	Cumulative rise 1990 to 2115 / metres (m)
East, East Midlands, London, South East	4 (140 mm)	8.5 (255 mm)	12 (360 mm)	15 (450 mm)	1.21 m
South West	3.5 (122.5 mm)	8 (240 mm)	11.5 (345 mm)	14.5 (435 mm)	1.14 m
North West, North East	2.5 (87.5 mm)	7 (210 mm)	10 (300 mm)	13 (390 mm)	0.99 m

- 2.1.9 The climate change guidance notes that the allowances provided have been derived from national scale research. There may be cases where local evidence supports the use of other local climate change allowances. With specific reference to changes to extreme rainfall LIT 5707 notes that UKCP09 provides useful information on change to rainfall across the UK.

## 2.2 Local Policies

### Thurrock Local Development Framework, Core Strategy and Policies for Management of Development. Development Plan Document Adopted December 2011

2.2.1 The Core Strategy is the central document of the Local Development Framework and sets out the vision, aims and strategy for spatial development in the Borough. It provides the framework for the formulation of the more detailed generic and site specific policies.

2.2.2 The policies relevant to hydrology are summarised in paragraphs 2.2.4 to 2.2.16

#### **Policy CSTP25 – Addressing Climate Change**

2.2.3 Developers must consider the potential effects of climate change on their development, including:

- water conservation and drainage; and
- flood risk from tidal, fluvial and surface water.

#### **CSTP27 – Management and Reduction of Flood Risk**

2.2.4 The Council will ensure that flood risk management is implemented and supported through effective land use planning. The Sequential, and where necessary Exception Test, as set out in PPS25 will be employed when allocating sites for development and an Emergency Plan for the Borough will be completed.

2.2.5 The Council will also continue to work collaboratively with the EA by supporting the area based policy approach adopted in the Thames Estuary 2100 Project. In particular the Council will seek to safeguard existing flood defences and new areas for flood defences, water storage and drainage areas, as well as seeking secondary defences for key assets.

2.2.6 The Council will support the work of the EA in the Environmental Enhancement Project for the Mucking Flats and Marshes to ensure the delivery of appropriate flood mitigation and environmental enhancement measures.

2.2.7 The Council will work with the EA and other main stakeholders to ensure that fluvial and surface water flood risk is managed within Thurrock. This will include supporting the policies identified in the South Essex Catchment Flood Management Plan, such as identifying and safeguarding areas of land for existing and future areas of water storage in Policy Units 9, 10, 11 & 12 and in formulating System Asset Management Plans (SAMP) and the Integrated Urban Drainage Plans for Stanford-le-Hope, Tilbury and Purfleet.

2.2.8 A Surface Water Management Plan will also be carried out to assist in the identification and mapping of areas susceptible to surface water flooding as recommended by Defra and the Pitt Review. Development proposals that will affect these locations will be expected to contribute towards infrastructure improvements in these locations to enable the development to proceed.

2.2.9 The Council will ensure that, where necessary, new development throughout the Borough contains space for water including naturalisation and environmental enhancement.

2.2.10 Developers will be required to contribute towards flood risk management infrastructure where appropriate.

2.2.11 Planning applications received for sites within Flood Zone 3 will be treated in accordance with PPS25, this policy and Policy PMD15.

#### **Policy PMD15 – Flood Risk Assessment**

2.2.12 Sites not covered by the Thurrock Sequential Test will be required to provide a site-specific Sequential Test to demonstrate compliance with PPS25 or any successor to be provided by the applicant. To reflect the nature of Thurrock's defended floodplain, particular reference should be made to the hazard rating for each site where covered by the Thurrock Strategic Flood Risk Assessment.

2.2.13 Only those applications classified under the 'minor development' or 'changes of use' categories will be exempt from applying the Sequential Test, but will still be expected to meet the requirements for Flood Risk Assessments and flood risk reduction as set out in Annex E of PPS25 and the associated Design and Sustainability Supplementary Planning Document.

2.2.14 Development proposals subject to the Exception Test in Thurrock must show that the following criteria have been met (in addition to FRA requirements outlined in PPS25):

- to assist with part a) of the Exception Test, reference should be made to the main assessment criteria outlined in the Thurrock Sustainability Appraisal and any



opportunities to reduce the overall flood risk posed to the community, including schemes to make space for water;

- the FRA must demonstrate that the development will be 'safe', without increasing flood risk elsewhere, and where possible will reduce flood risk overall. For Thurrock, this will mean addressing the following points in particular:
- flood hazard must be fully considered and reference should be made in the site-specific FRA to the SFRA, or site-specific modelling. This should be used to inform a sequential approach to planning within the site;
- where it is deemed acceptable to reduce flood storage as a result of development, level for level compensation storage must be provided to ensure that there is no increased flood risk elsewhere;
- where appropriate, an emergency plan for the development must be submitted that is consistent with the emergency plan for the area. This will include evidence that 'more vulnerable' development can achieve safe access/egress to a communal refuge point or unaffected area accessible to the emergency services. In highly exceptional cases where access/egress to a place of safe refuge cannot be achieved, these will be considered on their individual merits;
- where appropriate, flood avoidance, flood resistance and flood resilience measures must be incorporated into the design of any development;
- evidence that surface water management schemes, and other flood defence measures that are required on-site in order to allow a development to take place will be adequately maintained for the lifetime of that development by the site owner; and
- evidence that the proposed development will not interfere with the potential for future maintenance or improvements to flood defences.

2.2.15 Developers may be required to provide Developer Contributions towards the improvement of Emergency Planning services and flood defence measures within Thurrock as part of flood management mitigation.

2.2.16 Developments will be expected to incorporate Sustainable Drainage Systems (SUDS) to reduce the risk of surface water flooding, both to the site in question and to the surrounding area. Where the potential for surface water flooding has been identified, site specific Flood Risk Assessments should ensure that suitable SUDS techniques are incorporated as part of the redevelopment.

## 3. Site Setting

### 3.1 Site Location

- 3.1.1 The main development site comprises an irregular shaped parcel of land covering approximately 18.5 ha in Thurrock, Essex. The site is split into two distinct fields (north and south) by a land drainage network.
- 3.1.2 Further details of the site location and application boundary overall can be found in Volume 2, Chapter 2: Project Description of the Preliminary Environmental Information Report (PEIR).

### 3.2 Surrounding Area

- 3.2.1 The site sits within an area with mixed agricultural and industrial use, bound to the west and east by agricultural land, to the south by a National Grid substation with the River Thames and its associated mud and salt flats beyond, and to the north an embanked railway line with agricultural land beyond.

### 3.3 Existing Development

- 3.3.1 The site is presently utilised as agricultural land and Common Land with grazing rights.

### 3.4 Proposed Development

- 3.4.1 The application is for the development of a Flexible Generation Plant comprising the following within Zone A, with one possible layout shown in Figure 3.1.
- Gas engines, air pollutant control and cooling
  - Substations
  - Battery storage
  - Gas connection compound
  - Carbon Capture Ready area
  - Underground electrical cabling connecting to the National Grid substation
  - Internal access roads

- 3.4.2 Because the development layout within Zone A is subject to further design, as a maximum design scenario the FRA assumes that up to 17 ha of the 18.5 ha total Zone A site may be surfaced with low permeable materials (as some areas cannot be developed due to existing electricity pylons and overhead power lines) in order to present a conservative assessment of potential flood risk and mitigation design.

- 3.4.3 The proposed development is likely to be unmanned during the majority of the operational phase, being remotely operated and monitored with occasional access for maintenance.

### 3.5 Topographical Survey

- 3.5.1 A topographical survey of the site has been undertaken (Annex 15.1.1) which indicates that the north field has a gentle slope from northwest with levels c.1.4 m above Ordnance Datum (AOD) to the south east level c.1.23 mAOD.

The south field has general slope from west to east from 1.55 mAOD to 1.3 mAOD. However, there are numerous localised areas of elevated land with levels ranging from 1.6 mAOD to 1.8 mAOD.

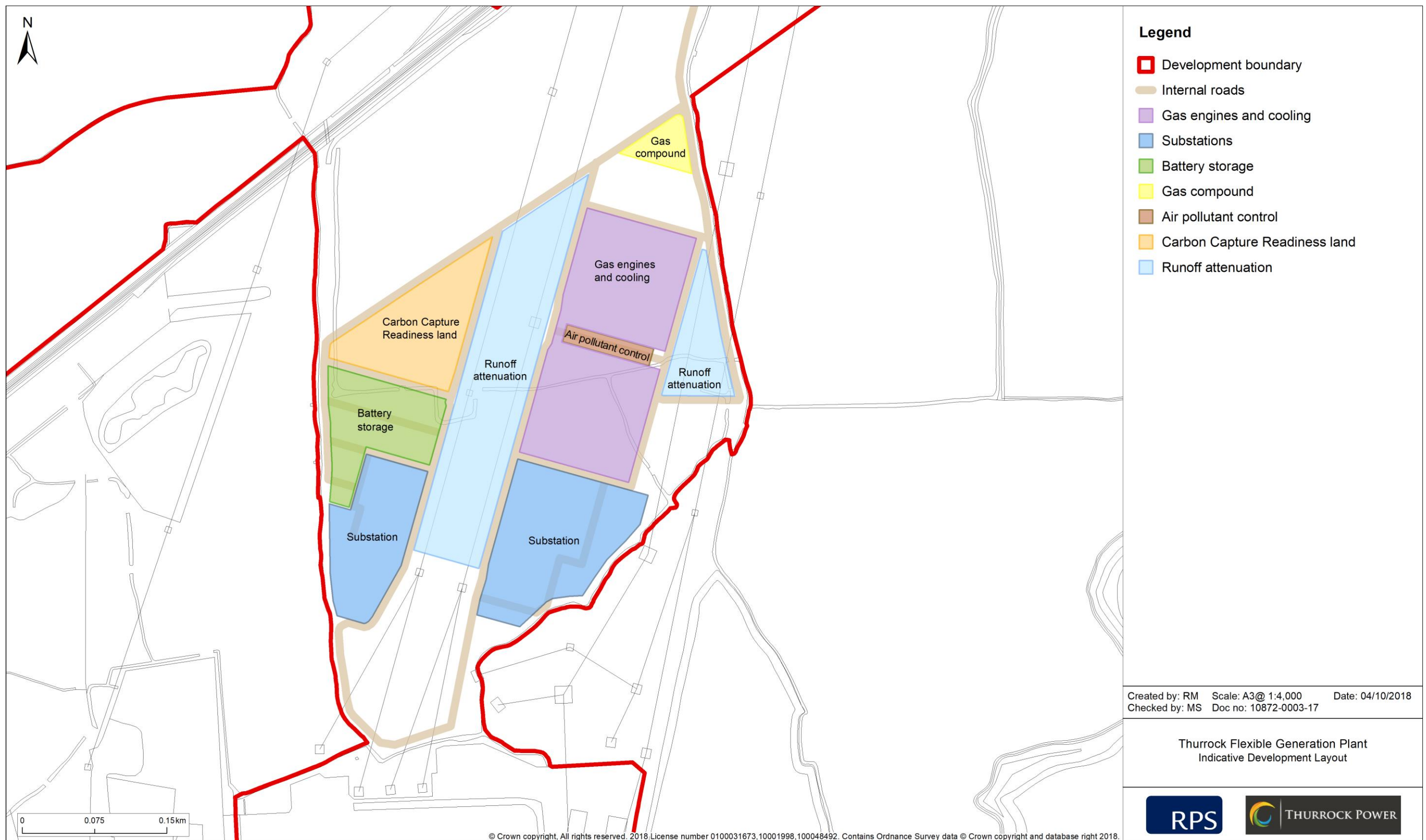


Figure 3.1: Indicative Zone A layout

## 4. Flood Risk Assessment

### 4.1 Hydrological overview

- 4.1.1 The closest watercourse to the site is the EA designated main River Thames.
- 4.1.2 The River Thames drains a catchment area of over 12,000 km<sup>2</sup>, tidally influenced for about 90 km of its length all the way up to the town of Teddington in Middlesex.
- 4.1.3 The Thames has posed a risk of flooding to London for as long as the settlement has been there. As London has grown, the river has become more and more constrained by the urban development. The natural floodplain of the River Thames within London is now almost fully developed and is heavily dependent upon manmade flood defences to protect it against the risk of flooding.
- 4.1.4 The southern boundary of Thurrock Council administrative boundary sits immediately adjacent to the River Thames. Historically, the River Thames floodplain in this area was substantially wider than it is today and the dense urban area of Greater London heavily constrains the passage of the river corridor as it winds its way towards the sea.
- 4.1.5 The River Thames has been heavily modified over time with the growth of London, including the construction of raised defences along much of its length. As a result, the direct risk to Tilbury as a result of fluvial flooding alone from the River Thames is negligible. OS mapping indicates that the site is surrounded by a number of unnamed drains, shallow ditches and water features. Surface water within the main body of the site flows in a general southerly and easterly direction discharging into West Tilbury Main c.500 m to the east, finally outfalling into the Thames.
- 4.1.6 The majority of the site drains into channels which outfall into open land drains which border the site, eventually discharging into the River Thames.

### 4.2 Flood Risk

- 4.2.1 The EA flood zone mapping (Figure 4.1 and Annex 15.1.2) indicates that the site is situated within undefended Flood Zone 2 and 3a, which is land assessed as having a 1 in 1,000 or greater annual probability of river and/or sea flooding.
- 4.2.2 EA Rivers and Sea flood mapping, which takes into account the effect of any flood defences that may be in this area, indicates the site is at low risk of fluvial or tidal flooding. Defined as areas with between 0.1% and 1% chance of flooding each year.

- 4.2.3 The Thurrock BC SFRA Flood Map indicates the site is located within Flood Zone 3a (Appendix: 15.2: Flood Zones and Model Data). However, is afforded flood protection by defence with a 1 in 1,000 year standard of protection (SoP).



Figure 4.1: EA Planning Flood Zone map (September 2018)

### Fluvial Flooding

- 4.2.4 EA data and previous flood reporting outlines that water levels in the Thames Estuary is dominated by tidal forces, with fluvial flows within the catchment generating a minimal impact. Extreme tidal water levels are far higher than those resulting from a major fluvial events, therefore fluvial flood risk is not considered any further.

### Tidal Flooding

- 4.2.5 No above ground permanent assets or ground profiling is proposed within areas of designated floodplain (Flood Zone 3b) located to the north of the railway line, as shown on Tilbury BC SFRA Level 2 flood zones (2109), reproduced in Volume 6, Appendix 15.2: Flood Zones and Model Data.

- 4.2.6 Taking into account the presence of tidal flood defence, which provide a SoP for events with up to a 1 in 1,000 year return period the tidal flood risk posed to the site is considered to be low.
- 4.2.7 Flood data extracted from the Thurrock BC SFRA Breach Modelling Study (2011) and the publicly available Tilbury 2 ES Appendix 16.B Level 3 Flood Risk Assessment presents a number of model simulations including a tidal breach within flood defences in close proximity to the proposed development. The results are summarised in Table 4.1.
- 4.2.8 The breach location was informed by Thurrock BC SFRA (see Volume 6, Appendix 15.2: Flood Zones and Model Data).

**Table 4.1: Summary of Breach model outputs (Tilbury2)**

Run period	Flood Depth (m)	Hazard Rating	Time to Inundation (hours)
1 in 200 year (2017)	0.5 – 1.0	Significant	4 to 12
1 in 200 year (2117)	0.5 – 1.5	Significant	1 to 8
1 in 1,000 year (2017)	0.5 – 1.5	Significant	N/A
1 in 1,000 year (2117)	0.5 – 1.5	Significant	N/A

- 4.2.9 The proposed development would incorporate flood resilience and/or resistance measures to ensure critical assets are afforded an appropriate level of flood protection.

### 4.3 Flood defences

- 4.3.1 The site is located within an area benefiting from flood defences (see Volume 6, Appendix 15.2: Flood Zones and Model Data). Areas benefiting from flood defences are defined as those areas which benefit from formal flood defences specifically in the event of flooding from rivers with a 1% (1 in 100) chance in any given year, or flooding from the sea with a 0.5% (1 in 200) chance in any given year.
- 4.3.2 EA records indicate that the standard of protection of the flood defences in close proximity to the site is 0.1% Annual Event Probability (AEP); designed to defend London up to a 1 in 1,000 year tidal flood event. The crest height of the flood defence wall protecting the site ranges from 6.48m AOD to 6.70m AOD.

- 4.3.3 The current condition grade for defences in the area ranges from fair to very poor, with the potential for severe defects resulting in complete performance failure, although it is noted that the Environment Agency has a duty to maintain these defences. The potential consequences of flood defence breach and design resilience measures have been assessed in Section 6.
- 4.3.4 Considerable investment has been made in the provision of the Thames Tidal Defences (TTD) to protect Greater London from tidal flooding. The tidal defences downstream of the Thames Barrier are maintained to a level of 7 mAOD, which, at the current time, provides a Standard of Protection (SoP) equivalent to the 0.1% AEP (1 in 1,000 year) tidal event.

### 4.4 Flooding from rising / high groundwater

- 4.4.1 Groundwater can be a problem at sites located on permeable strata, either superficial deposits such as river alluvium or bedrock such as fissured sandstone or limestone. Groundwater ingress can be a particular problem in alluvial sands and gravels during flooding where rising river levels cause rises in groundwater levels in deposits that are in hydraulic continuity with the river or flood water.
- 4.4.2 Geological mapping indicate that the site and surrounding area is underlain by Flandrian Alluvium comprising soft to firm consolidated, compressible silty clay, but can contain layers of silt, sand, peat and basal gravel. Seaford Chalk Formation and Newhaven Chalk Formation Composed of soft to medium hard, smooth white chalks with numerous marl seams and flint bands. A detailed assessment of the underlying geology is provided within Volume 3, Chapter 16: Geology, Hydrogeology and Ground Conditions.
- 4.4.3 Taking into account the above information and confidence limit the risk of groundwater flooding is considered to be low.

### 4.5 Source Protection Zones

- 4.5.1 EA mapping shows the site is not located within a Source Protection Zone.

## 4.6 Surface water flooding

Surface water flooding is caused by rainfall generated overland flow, before the runoff enters a watercourse or sewer. In such events sewerage and drainage systems and surface watercourses may be entirely overwhelmed. Surface water flood risk is assessed by simulating an extreme rainfall event over a 6.5 hours duration designed to completely overwhelm urban drainage systems, with 2D modelling used to identify overland flow routes and areas where surface water will pond.

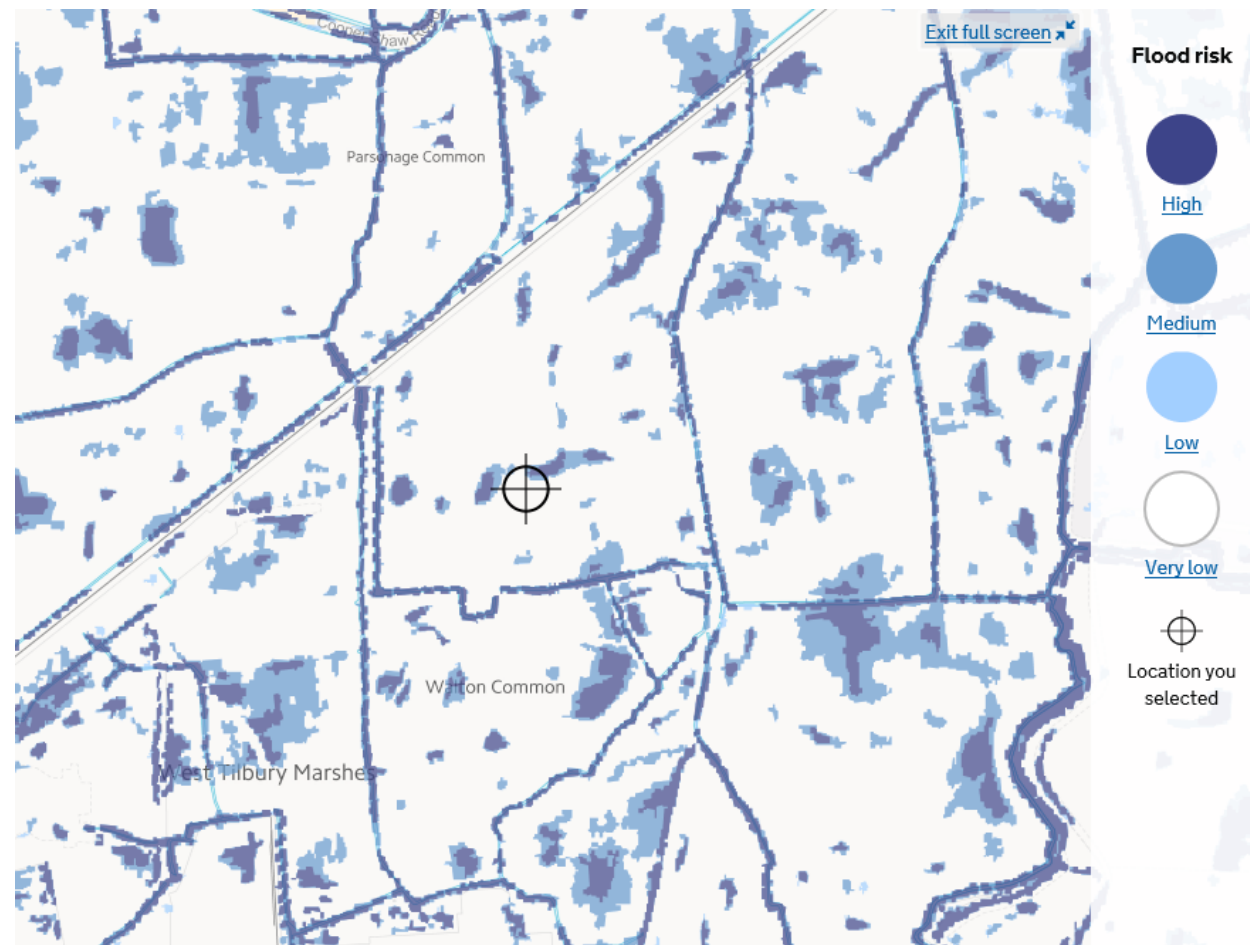


Figure 4.2: EA surface water risk map (September 2018).

- 4.6.1 EA mapping (Figure 4.2) indicates that the majority of the site is at very low risk of surface water flooding.
- 4.6.2 However, localised areas within the assessment area (in blue) are at low to high risk, defined as having between 1 in 1000 (0.1%) and 1 in 100 (1%) chance of flooding in any year, and areas with a greater than 3.3% chance of flooding in any year respectively. These areas have been assessed to be associated with localised areas of low lying land.

- 4.6.3 Based on EA mapping and site observations the overall surface water flood risk to the site has been assessed to be low.

## 4.7 Flood Warning Area

- 4.7.1 The site is located within a flood warning area. Full details on flood warning areas can be found at <https://www.fws.environment-agency.gov.uk/app/olr/login>.

## 4.8 Reservoir Failure Assessment

- 4.8.1 EA reservoir flood risk mapping shows that the site is not at risk of reservoir flooding.
- 4.8.2 The EA acknowledges that reservoir flood mapping indicates the 'worst case' extents and are very unlikely to happen, as this would require total loss of all water from the affected reservoir(s).
- 4.8.3 Generally the safety of the reservoir is the responsibility of the owners or operators who are required to maintain an adequate degree of protection. This is achieved by carrying out regular inspections, risk assessments and through the production and maintenance of Reservoir Flood Plans.

## 4.9 Sewer/Water Main Failure Assessment

- 4.9.1 No drainage records are available for the site. The land is currently agricultural land and therefore it is assumed that no artificial drainage systems will be present within the site area.
- 4.9.2 Under the DG 5 register requirements all water companies are obliged to keep a record of properties that have been affected by sewer flooding. The Thurrock BC SFRA historical drainage/sewer flooding map indicates that there are no incidents of sewerage flooding within the immediate area of the site.
- 4.9.3 Taking into account the above the overall risk of flooding from sewerage failure is assessed as low.

## 4.10 Infrastructure Failure Assessment

- 4.10.1 No flooding due to infrastructure failure has been recorded for the site.

## 4.11 Historical flood events

- 4.11.1 Thurrock BC SFRA notes that South Essex has been subject to two major flood events in 1928 and 1953. In January 1953, a major storm surge coincided with a high spring tide and resulted in the widespread flooding. Flood levels at Tilbury reached c.1.8 m above its predicted level and inundation depths were approximately 2-3 m.
- 4.11.2 The 1953 event led to the construction of an improved flood defence scheme at Purfleet, Grays, Tilbury, Tilbury Fort, Shell, Refinery, Canvey Island and the Holehaven and Benfleet barriers.

## 4.12 Present Flood Risk

- 4.12.1 The EA has confirmed that the site is located within Flood Zone 2 and 3, which is land assessed as having a 1 in 1,000 or greater annual probability of river and sea flooding.
- 4.12.2 Tidal flooding has been determined to pose the greatest risk to the site. Tidal defences are recorded to provide a SoP for tidal events with up to a 1 in 1,000 year return period. Therefore, the main flood risk to the site is as a consequence of a breach in local flood defences.
- 4.12.3 EA and SFRA mapping indicate the site is at flood risk due to fluvial flooding. The site is at low to medium risk of surface water and groundwater flooding.

## 5. Flood Risk Vulnerability Classification

### 5.1 Vulnerability Classification

- 5.1.1 In accordance with the Flood Risk Vulnerability Classification in Table 2 of the Planning and Practice Guidance Flood Risk and Coastal Change, Flexible Generation Plant is classified as 'Essential Infrastructure' development in flood risk terms.
- 5.1.2 The site is located within an area identified as Flood Zone 3a. Table 3 of Planning Practice Guidance (Table 5.1 of this report) indicates that Essential Infrastructure uses are acceptable for locations in Flood Zone 3a, but subject to an Exception and Sequential test.

- 5.2.2 When considering alternatives sites, the geographical location for the search area needs to consider the functional arrangements of the proposed development.
- 5.2.3 The site selection underpinning the development is presented in Volume 2, Chapter 3: Consideration of Alternatives.

Table 5.1: Flood Risk Vulnerability and Flood Zone 'Compatibility'

Flood Risk Vulnerability classification (see Table 3 of Planning Practice Guidance)	Essential Infrastructure	Water Compatible	Highly Vulnerable	More Vulnerable	Less Vulnerable
Zone 1	Yes	Yes	Yes	Yes	Yes
Zone 2	Yes	Yes	Exception test required	Yes	Yes
Zone 3a	Exception test required	Yes	No	Exception test required	Yes
Zone 3b Functional Floodplain	Exception test required	Yes	No	No	No

### 5.2 Sequential Test

- 5.2.1 The aim of the Sequential Test is to steer new development to areas with the lowest probability of flooding. As part of this process the developer will need to provide evidence to the Local Planning Authority that there are no other reasonably available sites where the development could be located.



## 6. Flood Management

### 6.1 Introduction

- 6.1.1 Section 4.2.6 notes that once the presence of existing tidal flood defences, which provide a 1 in 1,000 year SoP, are taken into account the fluvial and/or tidal risk is low.
- 6.1.2 Outputs from the Thurrock BC SFRA, which accounts for a worst case scenario, indicates that flood depths range across the site from 0.5 m to c.1.5 m for the 1 in 200 year plus climate change and for a 1 in 1,000 year event plus an allowance for climate change.
- 6.1.3 Taking into account the essential infrastructure development designation the below summarises potential mitigation options to provide flood protection for the 1 in 1,000 year plus climate change breach scenario. The options provided are not exhaustive and is presented as a guide to further consideration.

### 6.2 Mitigation Options

- 6.2.1 Mitigation options to reduce the exposure to the hazard and the vulnerability to the site after it has been developed, in line with Flood Risks to People, Phase 2; Improving the Flood Performance of New Buildings and Planning Practice Guidance ID7 are presented below.
- 6.2.2 Based on the potential flood depth (1.50 m), in line with the Improving the flood performance of new buildings (2007) document a Water Entry Strategy should be implemented to avoid any potential structural instability and to allow water passage through the fabric of the building.
- 6.2.3 To mitigate against the impacts of flooding on the development the proposed design and build would include the following;
- use of flood resistant and resilient materials where practicable; and
  - a flood evacuation plan.
- 6.2.4 Current EA guidance recommends that the minimum ground floor levels of new developments are set at the 1 in 200 annual probability plus allowance for climate change appropriate for the lifetime of the proposed development. Based on the essential nature of the proposed development a higher degree of protection may be required, which would be assessed on a cost-benefit basis by the applicant.

- 6.2.5 In the event that the development is unable to raise floor levels above the estimated flood level, extra flood resistance and resilience measures should be considered.
- 6.2.6 The exact details of which we would anticipated could be reasonably secured via a suitably worded requirement within the Development Consent Order.

### 6.3 Flood Protection Options

- 6.3.1 There are several flood risk management and mitigation measures that could be implemented to manage flood risk at the site, a selection of which are presented below.
- Water tight containers;
    - The proposed containerised power units and associated infrastructure will be designed so that they are water tight. This will prevent any ingress from flood waters and the containers / infrastructure will be secured so that flotation of the units would not occur during extreme flood events.
  - Raising assets; and
    - Raising assets above anticipated flood levels.
  - Bespoke building protection;
    - Elevating bunds surrounding transformers, cable ducts sealed with bentonite or with a fire resistant, waterproof system backing onto rubber insert sleeves. Air vents protected by a permanent rising flue. Doors reinforced and sealed, or building banded to provide appropriate protection.



Figure 6.1: Raised bund and access platform around a transformer



Figure 6.2: Veneer walling and access steps

## 6.4 Flood Warning

- 6.4.1 The EA aims to provide several hours' notice prior to the issue of a Flood Warning. It is recommended that the site operator registers on the EA Flood Warning System and implements on site management strategies to ensure that they can communicate flood warnings efficiently in order that the site can be evacuated when construction or maintenance staff are present.
- 6.4.2 It is recommended that a Flood Evacuation Plan is developed. Staff training would also be required, with the plan including information on what to do upon receipt of a flood warning, together with evacuation procedures and routes.
- 6.4.3 Flood Plans should be practiced regularly in order to minimise the risk to people.
- 6.4.4 Suitably trained staff would need to convey flood warning information and emergency procedures to occupants. All site personnel and contracted night security services would be required to be familiar with the flood action plan. Appropriate signage across the site (i.e. exits and assembly points) should be installed.
- 6.4.5 Additional information on the EA Flood Warnings and advice can be found on the EA's website (<https://fwd.environment-agency.gov.uk/app/olr/home>).

## 6.5 Safe Access/Egress

- 6.5.1 As noted in section 6.4 there is likely to be several hours warning before floodwater impact upon the site. Flood warning will be checked before a maintenance visit is conducted (<https://flood-warning-information.service.gov.uk/>).
- 6.5.2 If an occupant is on site there should be sufficient time to safely evacuate the site.
- 6.5.3 As flood water would propagate across the floodplain towards the site from the north it is proposed that occupants would evacuate the site on to Station Road and directed north towards West Tilbury Village Hall located within an area designated as Flood Zone 1.
- 6.5.4 The incorporation of flood resistance and resilience measures at the site, together with the implementation of a flood evacuation plan, would reduce the risk of damage and ensure the safety of occupants.

## 7. Drainage

### 7.1 Surface Water and Drainage Strategy

- 7.1.1 The sustainable management of surface water is an essential element of reducing future flood risk to the site and its surroundings.
- 7.1.2 Undeveloped sites generally rely on natural drainage to convey or absorb rainfall, the water soaking into the ground or flowing across the surface into watercourses.
- 7.1.3 The effect of development is generally to reduce the permeability of at least part of the site, which markedly changes the site's response to rainfall. Without specific measures to manage surface water the volume of water and peak flow rate are likely to increase. Inadequate surface water drainage arrangements can threaten the development itself and increase the risk of flooding to others.
- 7.1.4 Surface water arising from a developed site should as far as is practicable be managed in a sustainable manner to mimic the surface water flows arising from the site prior to the proposed development while reducing the risk of flooding at the site and elsewhere, taking climate change into account.

### 7.2 Sustainable Drainage Options

- 7.2.1 The NPPF and associated Planning Practice Guidance ID7, CIRIA C753 SUDS Manual (2015) and also Local Authority Policy promotes sustainable water management through the use of SuDS. A hierarchy of techniques is identified:
- Prevention – the use of good site design and housekeeping measures on individual sites to prevent runoff and pollution (e.g. minimise areas of hard standing).
  - Source Control – control of runoff at or very near its source (such as the use of rainwater harvesting).
  - Site Control – management of water from several sub-catchments (including routing water from roofs and car parks to one/several large soakaways for the whole site).
  - Regional Control – management of runoff from several sites, typically in a detention pond or wetland.
- 7.2.2 The implementation of SuDS as opposed to conventional drainage systems, provides several benefits by:

- reducing peak flows to watercourses or sewers and potentially reducing the risk of flooding downstream;
- reducing the volumes and frequency of water flowing directly to watercourses or sewers from developed sites;
- improving water quality over conventional surface water sewers by removing pollutants from diffuse pollutant sources;
- reducing potable water demand through rainwater harvesting;
- improving amenity through the provision of public open spaces and wildlife habitat; and
- replicating natural drainage patterns, including the recharge of groundwater so that base flows are maintained.

### 7.3 Surface Water Drainage Constraints

- 7.3.1 Constraints placed on the design of surface water drainage serving the proposed development are as follows:
- surface water drainage discharge methods will be limited to either soakaway drainage (if proven to be viable via infiltration testing) or discharge to watercourse. Discharge to public sewer is deemed unviable;
  - runoff from new development impermeable areas to be discharged at equivalent Greenfield runoff rates, requiring consideration of on-site surface water attenuation provision for extreme rainfall events; and
  - below ground electricity supply and distribution cabling associated with the development may impose restrictions on location and depth of below ground surface water drainage pipework runs.
- 7.3.2 Due to the site being remotely operated, and so potentially unoccupied for long periods of time, the proposed drainage system needs to provide resilience against occasional blockages or malfunction, and have minimal regular maintenance requirements.

### 7.4 Runoff Calculations

- 7.4.1 An assessment of the current and proposed runoff rates was undertaken to determine the surface water attenuation requirements for the site in line with The SuDS Manual (2015), which indicates that for previously developed sites the flow rate discharged from the site must not exceed that prior to the proposed development for the:
- 1 in 1 year event;

- 1 in 30 year event; and
- 1 in 100 year event.

7.4.2 The rates of runoff were determined using the current 'industry best practice' guidelines as outlined in the Interim Code of Practice for SuDS. The Defra/EA recommended methodology for sites up to 50 ha in area is the Institute of Hydrology Report 124 method (IoH 124). The runoff rates were calculated using the MicroDrainage software suite.

## 7.5 Current Runoff Rate

7.5.1 The site is presently covered by permeable agricultural land. The following parameters were incorporated into the existing site runoff calculations:

- Catchment Area: 17.00 ha (assume 100% permeable (landscaping) and 0% low permeable surfacing (hardstanding/buildings));
- Average Annual Rainfall (SAAR): 545 mm/year;
- Soil: 0.400;
- Region No: 6.

7.5.2 RPS has also calculated the runoff rate specific to the change low permeable surfacing (Table 7.1).

**Table 7.1: Runoff characteristics**

Annual Probability (Return Period, years)	Current Runoff (l/s)	Current Runoff (l/s/ha)
100% (1)	36.70	2.20
QBAR urban	43.20	2.50
3.33% (30)	97.80	5.80
1% (100)	137.70	8.10
1% + 40% Climate Change	192.78	11.34

Note: 40% added to rainfall data to account for long-term climate change

## 7.6 Attenuation Requirements

7.6.1 The attenuation volume required to restrict the surface water runoff rate from the additional low permeable surfacing to 36.7 l/s for a 1 in 100 year rainfall event plus climate change (+40%) has been determined using the industry standard MicroDrainage software suite incorporating the following parameters:

- Catchment Area: approximately 17.0 ha;
- Cv (proportion of rainfall forming surface water runoff): assume a factor of 75% for the development in summer, and 84% in winter (weighted average based on proposed land use);
- Runoff Rate: 36.7 l/s; and
- Assuming no infiltration losses.

7.6.2 The system was modelled within MicroDrainage as a tank/pond with controlled discharge via a hydrobrake outflow control. The MicroDrainage calculation sheets are included at Annex 15.1.3.

7.6.3 The attenuation volume required to restrict runoff to 36.7 l/s from a 1 in 100 year storm event plus a 40% allowance for climate change has been determined to be approximately 20,703 m<sup>3</sup> for the site.

## 7.7 Proposed Surface Water Drainage

7.7.1 The proposed new surface water drainage system will be designed using current MicroDrainage analysis software, to take account of planning guidance, Lead Local Flood Authorities and EA guidance to prevent uncontrolled flooding off the site to surrounding areas.

7.7.2 Surface water runoff from the proposed development will be collected as follows:

- permeable gravelled areas, unbound stone access roads and hardstanding – Runoff will be allowed to infiltrate naturally into the underlying geology; and
- impermeable building roof areas – traditional gravity gutters and downpipes, connected into a surface water attenuation system.

7.7.3 Surface water runoff will be collected and discharged to a below ground gravity the local surface water drainage network via an on-site surface water attenuation ponds totalling c.32,900 m<sup>2</sup>, designed to provide detention of flows during low intensity rainfall events and enough storage (c.20,703 m<sup>3</sup>) during extreme rainfall events to prevent uncontrolled flooding of the site. The pond will assist with the removal of sedimentation from rainwater runoff, and the downstream outlet of the pond will include a sump / catchpit for removal of silt and debris.

7.7.4 Whilst focusing on water quantity, the proposed system will also provide benefits in terms of water quality and biodiversity. A water quality risk assessment will be carried out using the SuDS hazard mitigation indices in accordance with Chapter 26, of the CIRIA C753 SuDS Manual.

- 7.7.5 Discharge of surface water from the site will be controlled to greenfield runoff rates by use of a vortex (Hydrobrake) flow control device fitted to the first upstream manhole from the surface water drainage outfall.
- 7.7.6 The discharge point will be an offsite existing watercourse to the east of the development site.
- 7.7.7 The location of the proposed attenuation can be seen on the Figure 3.1. Preliminary sizing of the attenuation pond has been calculated using the MicroDrainage software. From the calculations it is evident that the proposed storage is of sufficient size to accommodate runoff from the catchment area for all storms up to and including the 1 in 100 year rainfall event including a 40% allowance for climate change.

## 7.8 Construction Stage Drainage

- 7.8.1 During construction of the development, the building contractor will be responsible for management and disposal of rainwater runoff generated from the site in its temporary condition.
- 7.8.2 The contractor shall develop a formal site management plan, which will address pollution management and control in relation to site plant and vehicles, raw materials storage and waste generation, to ensure that all surface water runoff generated in the temporary condition will be free of contamination.
- 7.8.3 The site will be subject to topsoil strip and bulk earthworks to prepare the site to the correct level for development. The contractor shall provide temporary drainage measures to contain runoff within the development site boundary ensuring that this is sized appropriately, and that means to remove excess surface water are available for use at all times.
- 7.8.4 Surface water discharge from the site shall be passed to the receiving watercourse or soakaway ensuring that it is first passed via a silt filtration and removal device (Siltbuster) and that discharge will be via a controlled flow rate pump to the final receiving watercourse / soakaway at a rate no greater than the greenfield runoff rate for the site.

## 7.9 Water Quality / Pollution Control

- 7.9.1 The surface water drainage system will feature a number of SuDS measures that will be designed in accordance with CIRIA C753. As well as controlling the quantity of surface water run-off from the site, these features will also help to prevent the discharge of potential pollutants into the water environment downstream of the development.

## 8. Summary and Conclusions

### 8.1 Summary

8.1.1 A site-specific FRA in accordance with the NPPF and Planning Practice Guidance has been undertaken for the proposed gas engine, battery storage and substation elements associated with the Thurrock Flexible Generation Plant on a parcel of land off Station Road, Tilbury, Thurrock.

### 8.2 Flood Risk

8.2.1 EA and Thurrock BC SFRA mapping shows that the proposed gas engine, battery storage and substation (Zone A) is entirely located in Flood Zone 3a and is consequently at 'high' risk of flooding.

8.2.2 EA records identify that the proposed development is protected by flood defences with a SoP of in excess of 1 in 1,000 years.

8.2.3 Based on breach model outputs supplied within the Thurrock SFRA and Tilbury 2 report for the 'worst case' 1 in 1,000 year event the residual flood risk to the proposed development as a consequence of defence failure ranges from 0.5 to 1.5 m flood depth.

8.2.4 SFRA records indicate the sites flooded most recently in January 1953, due to a major storm surge coincided with a high spring tide.

8.2.5 The site majority of the site is located at low susceptibility of surface water flooding. Isolated areas of low lying land are at medium to high risk of surface water flooding.

8.2.6 The susceptibility to groundwater flooding is low.

8.2.7 SFRA mapping indicate that no sewerage flooding incidents have been reported in close proximity to the site. The risk of flooding from sewerage failure is therefore considered to be low.

8.2.8 The risk of flooding from reservoir failure has been assessed as low.

8.2.9 The attenuation volume required to restrict runoff for low permeable surfacing to the greenfield 36.7 l/s runoff rate from a 1 in 100 year storm event plus a 40% allowance for climate change has been determined to be approximately 20,703 m<sup>3</sup> for the site.

### 8.3 Conclusion

8.3.1 This FRA and supporting documentation illustrates that the application area is at risk of flooding from tidal sources, but meets the requirements of the NPS's, NPPF and Planning Practice Guidance.

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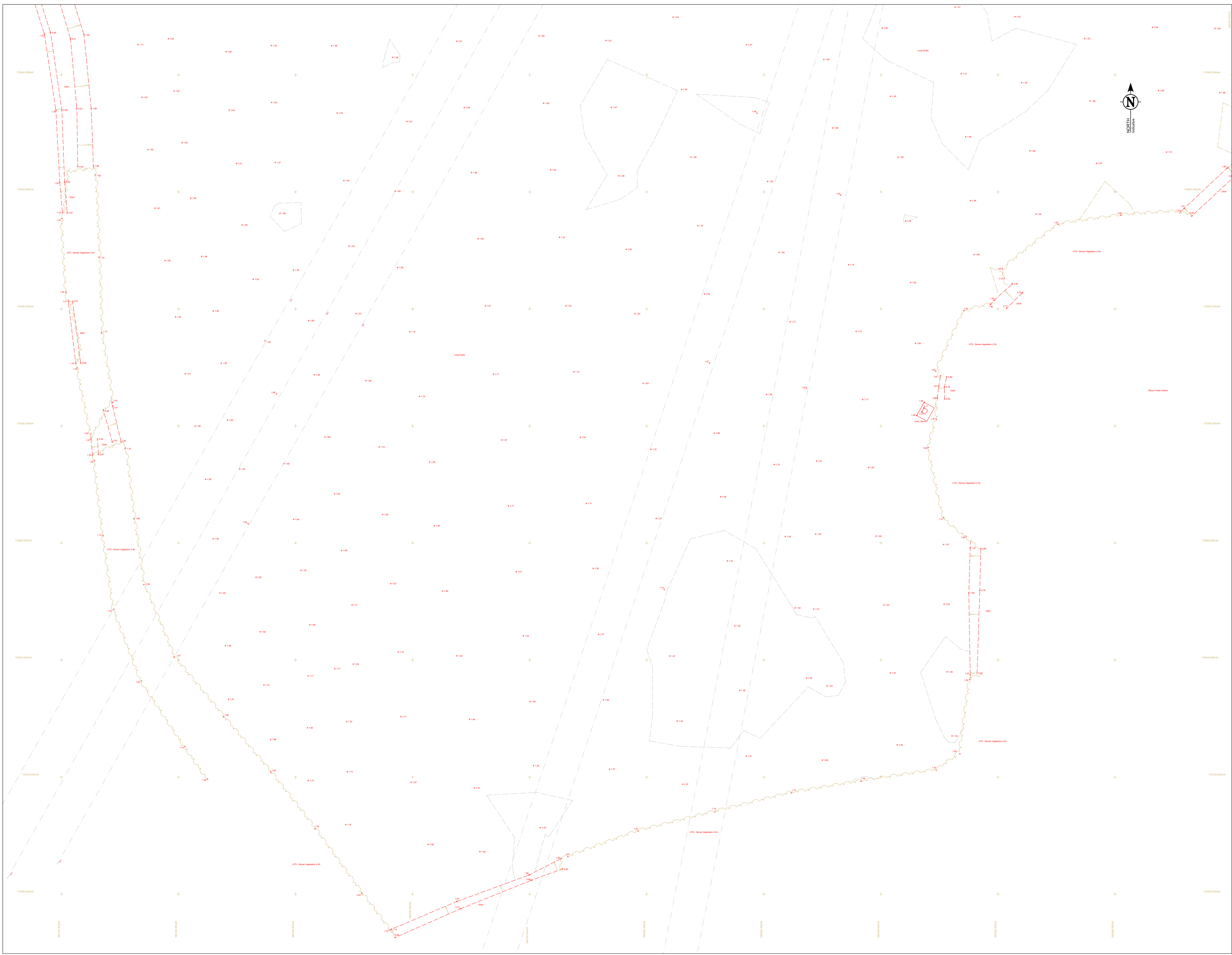
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## Annex 15.1.1: Topographical survey





**TOPOGRAPHICAL & MEASURED BUILDING SURVEYS**

**ABBREVIATIONS & SYMBOLS**

AH	Arch Head Height	FC	Firm Footing	RSU	Revised Level
AL	Assumed Level	FSD	Floor Slab Depth	SL	Sign Post
AV	Air Valve	FD	Floor Drain	SP	Site Sign
BA	Balustrade	FL	Floor Level	SV	Site Valve
BS	Bore Hole	GD	Grid Datum	SURF	Surface
BL	Bolt Level	PW	Post Wall	SY	Site
BO	Bolt	GC	Grid Centre	TAR	Tar
BP	Bench Point	GV	Gas Valve	TC	Tarmac Cover
BS	Bus Stop	HM	Hard Mastic	TFL	Threatened Level
BU	Bush	IC	Imperial Cover	TL	Traffic Light
BW	Barbed Wire Fence	L	Level	TS	Traffic Sign
BS	Bull Bars	LS	Level Sign	TW	Turf Wall
CB	Close Board Fence	ASD	Arch Slab	TP	Top of Pole
CD	Cable Duct	LD	Level Datum	UL	Under Level
CH	Chimney	LP	Lamp Post	UV	Under Floor
CL	Chain Link Fence	MB	Master Post	UC	Under Chamber
CL	Chain Link	NB	Narrow Band	UL	Under Level
CO	Column	OPL	Overhead Line Pole	US	Under Side
COP	Concrete Paving Panel	PO	Post	USB	Under Side Beam
CR	Cable Race	PS	Pipe	W	Wall
CC	Change Current	PP	Post Pole	WB	Water Basin
DC	Door Case	PR	Post & Rail	WR	Water Race
DD	Door Head	PR	Post & Rail Face	WS	Water Sign
DL	Drain	PW	Post & Wire Fence	WW	Water Well
EL	Electric Pole	RM	Ridge Ridge	WD	Water Drain
EP	Earth Pipe	RS	Roof Ridge	WF	Water Flow
ER	Earth Road	RS	Ridge Ridge	WF	Water Flow
ET	ET Transformer	RP	Rail Post	WF	Water Flow
FB	Floor Bed	RSD	Rail Sign	WF	Water Flow
FD	Floor Slab Depth	RSD	Rail Sign	WF	Water Flow

**DRAWING NOTE**

**Topographical Surveys**  
 These are drawn to scale showing the average canopy spread. Descriptions and heights should be used as a guide only.

All building names, descriptions, number of storeys, construction type including roof line details are indicative only and taken externally from ground level.  
 All below ground details including drainage, voids and services have been identified from above ground and therefore all details relating to these features including: access, depth, descriptions etc will be approximate only. All critical dimensions and connections should be checked and verified prior to starting work.

Detail, services and features may not have been surveyed if obstructed or not reasonably visible at the time of the survey.

**Measured Building Surveys**  
 Measurements to internal walls are taken to the wall finishes at approx 1m above the floor level and the wall assumed to be vertical.  
 All heights are measured as floor to the sill and head heights are measured from sill to the top of window.

**General**  
 The contractor must check and verify all site and building dimensions, levels, utilities and drainage details and connections prior to commencing work. Any errors or discrepancies must be notified to Survey Solutions immediately.

The accuracy of the digital data is the same as the plotting scale implies. All dimensions are in metres unless otherwise stated.

The survey control used is only to be used for topographical surveys at the stated scale. All control must be checked and verified prior to use.

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Do not scale from this drawing.

**CONTROL CO-ORDINATES**

STATIONS	EASTINGS	NORTHINGS	LEVEL	DESCRIPTION
1701	56619.600	17742.187	1.663	Page 4 Nail
1702	56628.990	17705.888	1.722	Page 4 Nail
1703	56629.428	17678.142	1.675	Page 4 Nail
1704	56614.828	17658.131	1.658	Page 4 Nail

**SURVEY GRID AND LEVEL DATUM**

The co-ordinate system established for this survey is related to Ordnance Survey (OS) national grid at a single point using GPS Smartnet, then orientated to Grid North with a scale factor of 1.000.

The level datum established for this survey is related to Ordnance Survey (OS) using GPS Smartnet.

To avoid discrepancies, any co-ordinated data used in conjunction with this survey must be derived directly from this control data.

The major contour interval is 1 metre, the minor contour interval is 0.500 metres.

Please note snow was present on the ground at the time of the survey which may have obstructed details.

REV	DESCRIPTION	DRAWN	APPR	DATE

**SURVEY SOLUTIONS**

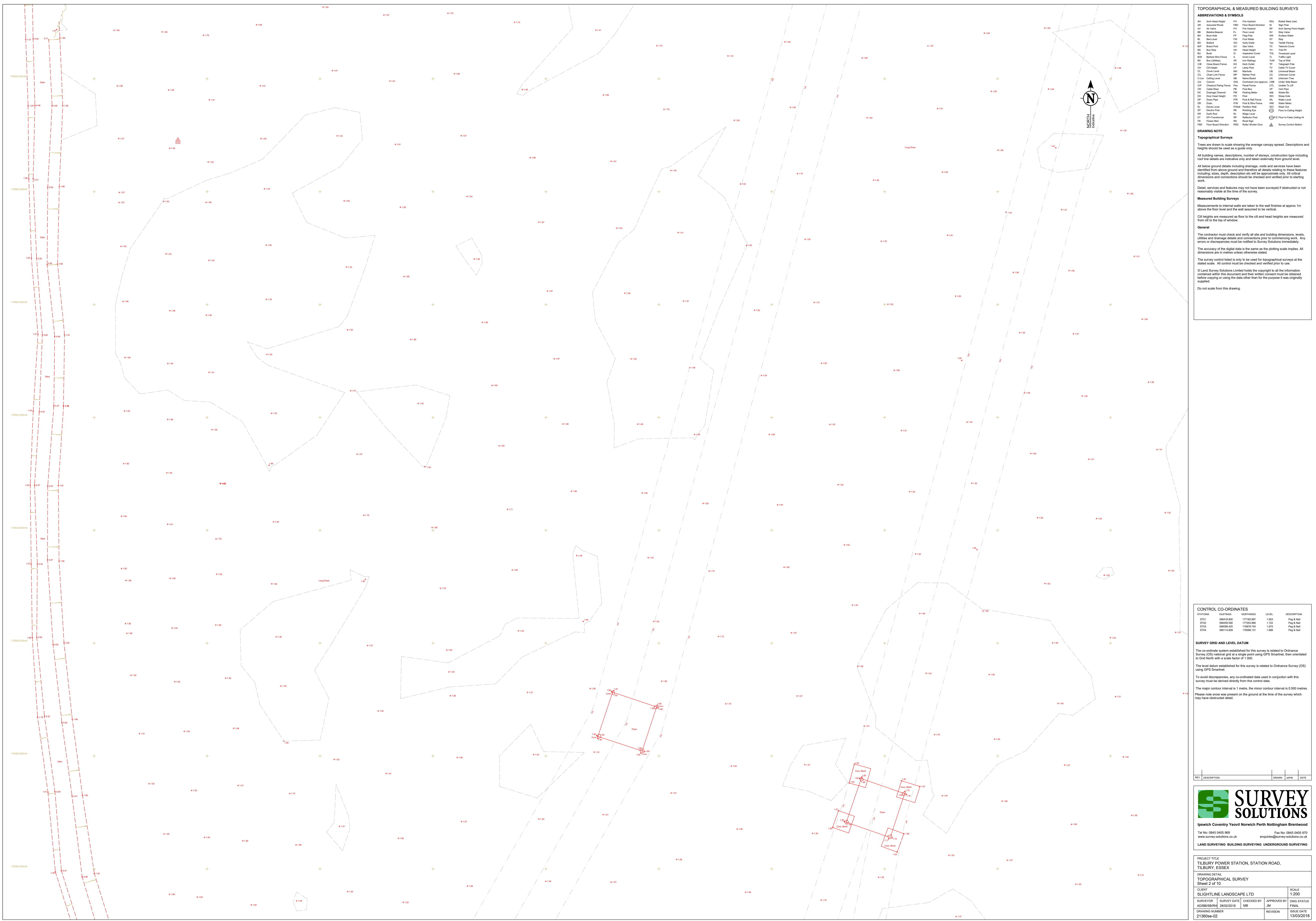
Ipswich Coventry Yeovil Norwich Perth Nottingham Brentwood  
 Tel No: 0845 9405 969 Fax No: 0845 0405 970  
 www.survey-solutions.co.uk enquiries@survey-solutions.co.uk

**LAND SURVEYING BUILDING SURVEYING UNDERGROUND SURVEYING**

PROJECT TITLE  
 TILBURY POWER STATION, STATION ROAD,  
 TILBURY, ESSEX

DRAWING DETAIL  
 TOPOGRAPHICAL SURVEY  
 Sheet 1 of 10

CLIENT	SLIGHTLINE LANDSCAPE LTD	SCALE	1:200
SURVEYOR	JM	APPROVED BY	DWG STATUS
ADVISOR/CHK	28/02/2018	CHECKED BY	FINAL
DRAWING NUMBER	213603a-01	REVISION	ISSUE DATE
			13/03/2018



**TOPOGRAPHICAL & MEASURED BUILDING SURVEYS**

**ABBREVIATIONS & SYMBOLS**

AN	Architect Height	AS	Area	BSU	Buffer Stop
AR	Asphalt Road	BD	Basin	BT	Bell
AV	Asphalt	BE	Beam	BT	Black Top
BB	Ballast Bed	FL	Floor Level	BT	Black Top
BH	Burn Hole	GD	Gravel	BT	Bell Top
BL	Ball Level	FW	Foot Wall	ST	Step
BO	Bolt	GD	Gravel	TA	Table
BP	Base Point	GV	Gull Valve	TC	Tarmac
BS	Back Step	IC	Impedance	TL	Tarmac
BU	Back Step	IC	Impedance	TL	Tarmac
BV	Barbed Wire Fence	L	Level	TL	Tram Line
BS	Bus Stop	LS	Low Siding	TW	Top of Wall
CB	Cable Box	ND	North	TW	Top of Wall
CC	Cable Channel	LP	Lamp Post	TV	Cable TV Core
CD	Clearance	M	Mast	UB	Under Bench
CE	Chain Level	MP	Manhole	UC	Under Cover
CF	Chain Level	MS	Manhole	UC	Under Cover
CG	Chain Level	MS	Manhole	UC	Under Cover
CH	Chain Level	MS	Manhole	UC	Under Cover
CI	Chain Level	MS	Manhole	UC	Under Cover
CL	Chain Level	MS	Manhole	UC	Under Cover
CM	Chain Level	MS	Manhole	UC	Under Cover
CO	Chain Level	MS	Manhole	UC	Under Cover
CP	Chain Level	MS	Manhole	UC	Under Cover
CQ	Chain Level	MS	Manhole	UC	Under Cover
CR	Chain Level	MS	Manhole	UC	Under Cover
CS	Chain Level	MS	Manhole	UC	Under Cover
CT	Chain Level	MS	Manhole	UC	Under Cover
CU	Chain Level	MS	Manhole	UC	Under Cover
CV	Chain Level	MS	Manhole	UC	Under Cover
CW	Chain Level	MS	Manhole	UC	Under Cover
CX	Chain Level	MS	Manhole	UC	Under Cover
CY	Chain Level	MS	Manhole	UC	Under Cover
CZ	Chain Level	MS	Manhole	UC	Under Cover
DA	Ditch	FD	Floor Finish	WB	Water
DB	Ditch	FD	Floor Finish	WB	Water
DC	Ditch	FD	Floor Finish	WB	Water
DD	Ditch	FD	Floor Finish	WB	Water
DE	Ditch	FD	Floor Finish	WB	Water
DF	Ditch	FD	Floor Finish	WB	Water
DG	Ditch	FD	Floor Finish	WB	Water
DH	Ditch	FD	Floor Finish	WB	Water
DI	Ditch	FD	Floor Finish	WB	Water
DJ	Ditch	FD	Floor Finish	WB	Water
DK	Ditch	FD	Floor Finish	WB	Water
DL	Ditch	FD	Floor Finish	WB	Water
DM	Ditch	FD	Floor Finish	WB	Water
DN	Ditch	FD	Floor Finish	WB	Water
DO	Ditch	FD	Floor Finish	WB	Water
DP	Ditch	FD	Floor Finish	WB	Water
DQ	Ditch	FD	Floor Finish	WB	Water
DR	Ditch	FD	Floor Finish	WB	Water
DS	Ditch	FD	Floor Finish	WB	Water
DT	Ditch	FD	Floor Finish	WB	Water
DU	Ditch	FD	Floor Finish	WB	Water
DV	Ditch	FD	Floor Finish	WB	Water
DW	Ditch	FD	Floor Finish	WB	Water
DX	Ditch	FD	Floor Finish	WB	Water
DY	Ditch	FD	Floor Finish	WB	Water
DZ	Ditch	FD	Floor Finish	WB	Water
EA	Earth	FE	Floor Finish	WB	Water
EB	Earth	FE	Floor Finish	WB	Water
EC	Earth	FE	Floor Finish	WB	Water
ED	Earth	FE	Floor Finish	WB	Water
EE	Earth	FE	Floor Finish	WB	Water
EF	Earth	FE	Floor Finish	WB	Water
EG	Earth	FE	Floor Finish	WB	Water
EH	Earth	FE	Floor Finish	WB	Water
EI	Earth	FE	Floor Finish	WB	Water
EJ	Earth	FE	Floor Finish	WB	Water
EK	Earth	FE	Floor Finish	WB	Water
EL	Earth	FE	Floor Finish	WB	Water
EM	Earth	FE	Floor Finish	WB	Water
EN	Earth	FE	Floor Finish	WB	Water
EO	Earth	FE	Floor Finish	WB	Water
EP	Earth	FE	Floor Finish	WB	Water
EQ	Earth	FE	Floor Finish	WB	Water
ER	Earth	FE	Floor Finish	WB	Water
ES	Earth	FE	Floor Finish	WB	Water
ET	Earth	FE	Floor Finish	WB	Water
EU	Earth	FE	Floor Finish	WB	Water
EV	Earth	FE	Floor Finish	WB	Water
EW	Earth	FE	Floor Finish	WB	Water
EX	Earth	FE	Floor Finish	WB	Water
EY	Earth	FE	Floor Finish	WB	Water
EZ	Earth	FE	Floor Finish	WB	Water
FA	Fence	FR	Floor Finish	WB	Water
FB	Fence	FR	Floor Finish	WB	Water
FC	Fence	FR	Floor Finish	WB	Water
FD	Fence	FR	Floor Finish	WB	Water

**DRAWING NOTE**

**Topographical Surveys**  
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**CONTROL CO-ORDINATES**

STATION	EASTING	NORTHING	LEVEL	DESCRIPTION
ST01	56618.600	17703.997	1.001	Peg & Nail
ST02	56629.000	17702.866	1.722	Peg & Nail
ST03	56609.425	17697.765	1.025	Peg & Nail
ST04	56614.828	17698.131	1.693	Peg & Nail

**SURVEY GRID AND LEVEL DATUM**


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 Please note snow was present on the ground at the time of the survey which may have obstructed details.

REV	DESCRIPTION	DRAWN	APPROVED	DATE



**SURVEY SOLUTIONS**

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LAND SURVEYING BUILDING SURVEYING UNDERGROUND SURVEYING

PROJECT TITLE TILBURY POWER STATION, STATION ROAD, TILBURY, ESSEX	SCALE 1:200
DRAWING DETAIL TOPOGRAPHICAL SURVEY Sheet 2 of 10	CHECKED BY JM
CLIENT SLIGHTLINE LANDSCAPE LTD	APPROVED BY DWS
SURVEYOR ADAM BURTON	DATE 20/02/2018
DRAWING NUMBER 213609a-02	REVISION 1
	ISSUE DATE 13/03/2018





TOPOGRAPHICAL & MEASURED BUILDING SURVEYS

ABBREVIATIONS & SYMBOLS

AL	Asphalt	FL	Flag	RL	Roof Level
AR	Asphalt Road	FSD	Floor Slab Detail	SI	Sign Post
AV	Air Valve	FH	Fire Hydrant	SP	Arch Span Post Height
BB	Ballast Bed	FL	Floor Level	SV	Site Valve
BR	Bar Bed	GC	Gully Cover	SW	Surface Water
BL	Basin Level	FW	Foot Wall	SY	Sy
BS	Built	IC	Insulation Cover	TF	Turf
BP	Base Post	GV	Gull Valve	TC	Taken Cover
BS	Base Stop	HW	Head Work	TR	Tree
BS	Base Stop	IC	Insulation Cover	TL	Thatched Level
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light
BS	Base Stop	IC	Insulation Cover	TL	Turf Light

**DRAWING NOTE**

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CONTROL CO-ORDINATES

STATION	EASTING	NORTHINGS	LEVEL	DESCRIPTION
ST01	56614.820	17695.887	1.623	Page & Nail
ST02	56625.290	17702.986	1.722	Page & Nail
ST03	56639.429	17695.164	1.675	Page & Nail
ST04	56614.828	17698.131	1.668	Page & Nail

**SURVEY GRID AND LEVEL DATUM**

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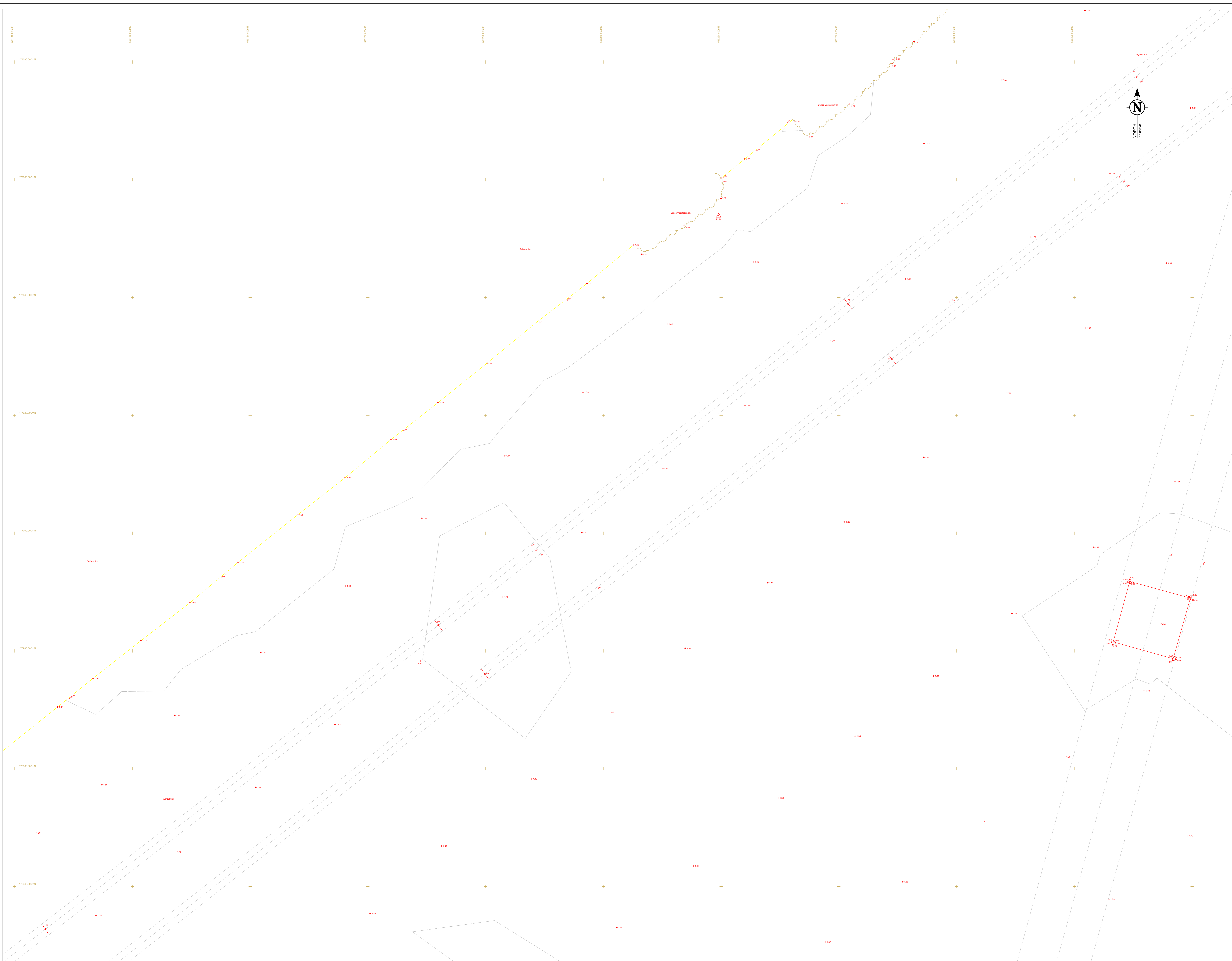
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**LAND SURVEYING BUILDING SURVEYING UNDERGROUND SURVEYING**

PROJECT TITLE  
TILBURY POWER STATION, STATION ROAD,  
TILBURY, ESSEX

DRAWING DETAIL  
TOPOGRAPHICAL SURVEY  
Sheet 4 of 10

CLIENT SLIGHTLINE LANDSCAPE LTD	SCALE 1:200			
SURVEYOR ADM/SUR/21	SURVEY DATE 28/03/2018	CHECKED BY JM	APPROVED BY DWS	STATUS FINAL
DRAWING NUMBER 213603e-04	REVISION	ISSUE DATE 13/03/2018		



### TOPOGRAPHICAL & MEASURED BUILDING SURVEYS

**ABBREVIATIONS & SYMBOLS**

Air	Arch Head Height	AG	Grid Point	RSL	Roof Level
AK	Asphalt Road	FBD	Floor Board Direction	SL	Skip Foot
AK	Asphalt	FR	Foot Rule	SP	Arch Spring Head Height
BA	Ballina Beams	FL	Floor Level	SV	Side View
BS	Bolt Hole	GC	Gully Cover	SW	Surface Water
BL	Belt Level	FW	Foot Wall	SY	Step
BND	Band	QD	Quarry	TA	Traffic
BR	Braze Plate	GV	Gully View	TC	Tarmac Cover
BR	Brick Base	HC	Head Height	TL	Tram Line
BU	Brick Slip Fence	IC	Insulation Cover	TL	Tram Level
BW	Barbed Wire Fence	L	Level	TS	Traffic Light
BK	Black (Kilns)	LS	Low Railings	TW	Turf Wall
CB	Cable Board Fence	ND	Nail	TP	Telephone Pole
CC	Cable Level	LH	Lang Post	TV	Cable TV Cover
CC	Cable Level	MS	Master Plate	UB	Underground
CC	Chain Link Fence	MP	Master Plate	UC	Underground
CD	Cable Level	NS	Northward	UC	Underground
CL	Column	OM	Overhead Line (Open)	USD	Under Side Beam
CP	Chamber Paving Pattern	PN	Panel Stone	US	Under Side
CR	Cable Road	PS	Post	VN	Vent Pipe
DC	Change Channel	PR	Printing Paper	WB	Water Bolt
DI	Door Head Height	PD	Post	WN	Wing Wall
DJ	Door Post	PIB	Post & Rail Fence	WS	Water Seal
DR	Drain	PIW	Post & Wire Fence	WW	Water Wall
DL	Down Level	PIW	Post & Wire Fence	WD	Water Down
DN	Drain Pipe	PJH	Post & Rail Fence	WO	Water Out
DP	Drain Pipe	RE	Reading Eye		
EP	Earth Pit	RL	Ridge Level		
ET	EP Transformer	RP	Rubber Post		
FB	Foot Path	RSD	Rubber Shutter Door		
FD	Floor Board Direction	RSD	Rubber Shutter Door		

**DRAWING NOTE**

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### CONTROL CO-ORDINATES

STATION	EASTINGS	NORTHINGS	LEVEL	DESCRIPTION
ST01	566203.000	177022.000	1.663	Page & Nail
ST02	566209.000	177022.000	1.722	Page & Nail
ST03	566209.425	176998.425	1.655	Page & Nail
ST04	566114.828	176998.121	1.658	Page & Nail

**SURVEY GRID AND LEVEL DATUM**


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REV	DESCRIPTION	DRAWN	APP'D	DATE



**SURVEY SOLUTIONS**

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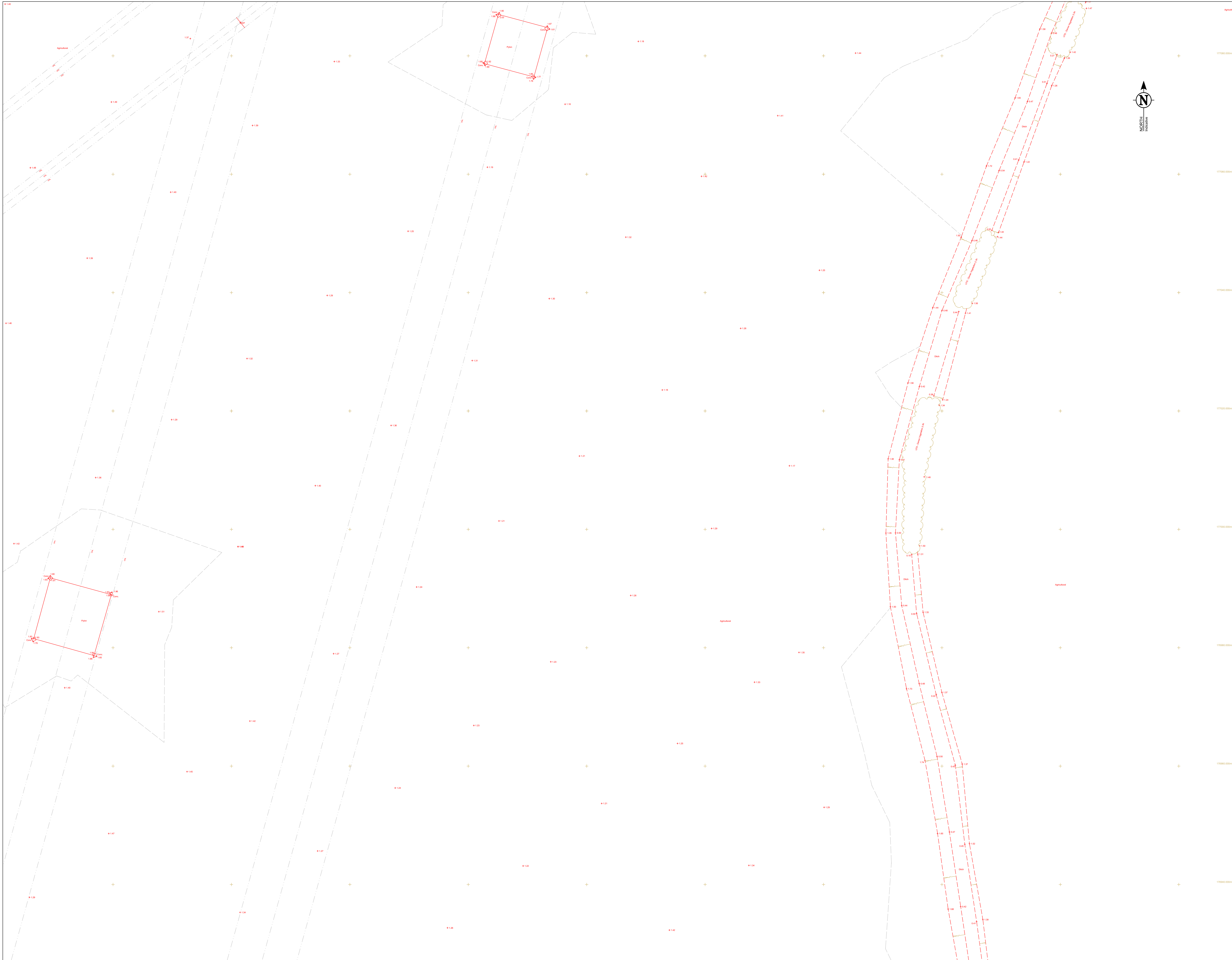
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**LAND SURVEYING BUILDING SURVEYING UNDERGROUND SURVEYING**

PROJECT TITLE TILBURY POWER STATION, STATION ROAD, TILBURY, ESSEX			
DRAWING DETAIL TOPOGRAPHICAL SURVEY Sheet 5 of 10			
CLIENT SLIGHTLINE LANDSCAPE LTD	SCALE 1:200		
SURVEYOR ADAMS/SHW	SURVEY DATE 28/03/2018	CHECKED BY JM	APPROVED BY PJK
DRAWING NUMBER 213603a-05	REVISION	ISSUE DATE 13/03/2018	

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**TOPOGRAPHICAL & MEASURED BUILDING SURVEYS**

ABBREVIATIONS & SYMBOLS					
AN	Actual Height	FD	Fine Finish	RSU	Right Hand Side
AR	Asurveyed Route	FD	Floor Slab Detail	SF	Sign Post
AV	Air Valve	FD	Floor Slab	SP	Sign Post Height
BA	Balance Beam	FL	Floor Level	SV	Site Valve
BR	Brick Base	GC	Grid Contour	SW	Surface Water
BL	Bull Level	FW	Foot Wall	SY	Sky
BD	Bolt	IC	Internal Corner	TW	Trade Window
BP	Brace Post	GV	Gas Valve	TC	Taken Cover
BS	Bulkhead	HT	Hand Height	TR	Trade
BU	Bulkhead Face	IC	Internal Corner	TL	Trade Level
BU	Bulkhead Face	L	Level	TLS	Trade Level Sign
BS	Bulkhead Face	US	Under Surface	TW	Trade Window
CB	Close Board Fence	ACD	Architrave	TT	Top of Wall
CA	Cable	LP	Lamp Post	TV	Cable TV Cover
CA	Close Board Fence	MS	Manhole	UB	Under Board
CC	Close Board Fence	MP	Manhole Post	UC	Under Cover
CCL	Cable Level	NB	New Board	UL	Under Level
CD	Close Board Fence	ODS	Overhead Line Support	USB	Under Side Beam
CP	Close Board Fence	PP	Pipe	UTL	Under Top Level
CR	Close Board Fence	PS	Post	VF	Vegetation
CC	Close Board Fence	PK	Post	WB	Water Basin
DR	Close Board Fence	PR	Post & Rail Fence	WV	Water Valve
DL	Close Board Fence	PF	Post & Wire Fence	WL	Water Level
DL	Close Board Fence	PM	Post & Wire Fence	WV	Water Valve
EP	Close Board Fence	RE	Rising Entry	WV	Water Valve
EP	Close Board Fence	RH	Rising Head	WV	Water Valve
EP	Close Board Fence	RF	Rising Foot	WV	Water Valve
EP	Close Board Fence	RF	Rising Foot	WV	Water Valve
EP	Close Board Fence	RF	Rising Foot	WV	Water Valve
EP	Close Board Fence	RF	Rising Foot	WV	Water Valve
EP	Close Board Fence	RF	Rising Foot	WV	Water Valve

**DRAWING NOTE**

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CONTROL CO-ORDINATES			
STATION	EASTINGS	NORTHINGS	LEVEL DESCRIPTION
3101	98814.830	177023.887	1.623
3102	98828.390	177028.386	1.722
3103	98828.425	17678.142	1.675
3104	98814.832	17668.131	1.688

**SURVEY GRID AND LEVEL DATUM**

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REV.	DESCRIPTION	DRAWN	APPR.	DATE



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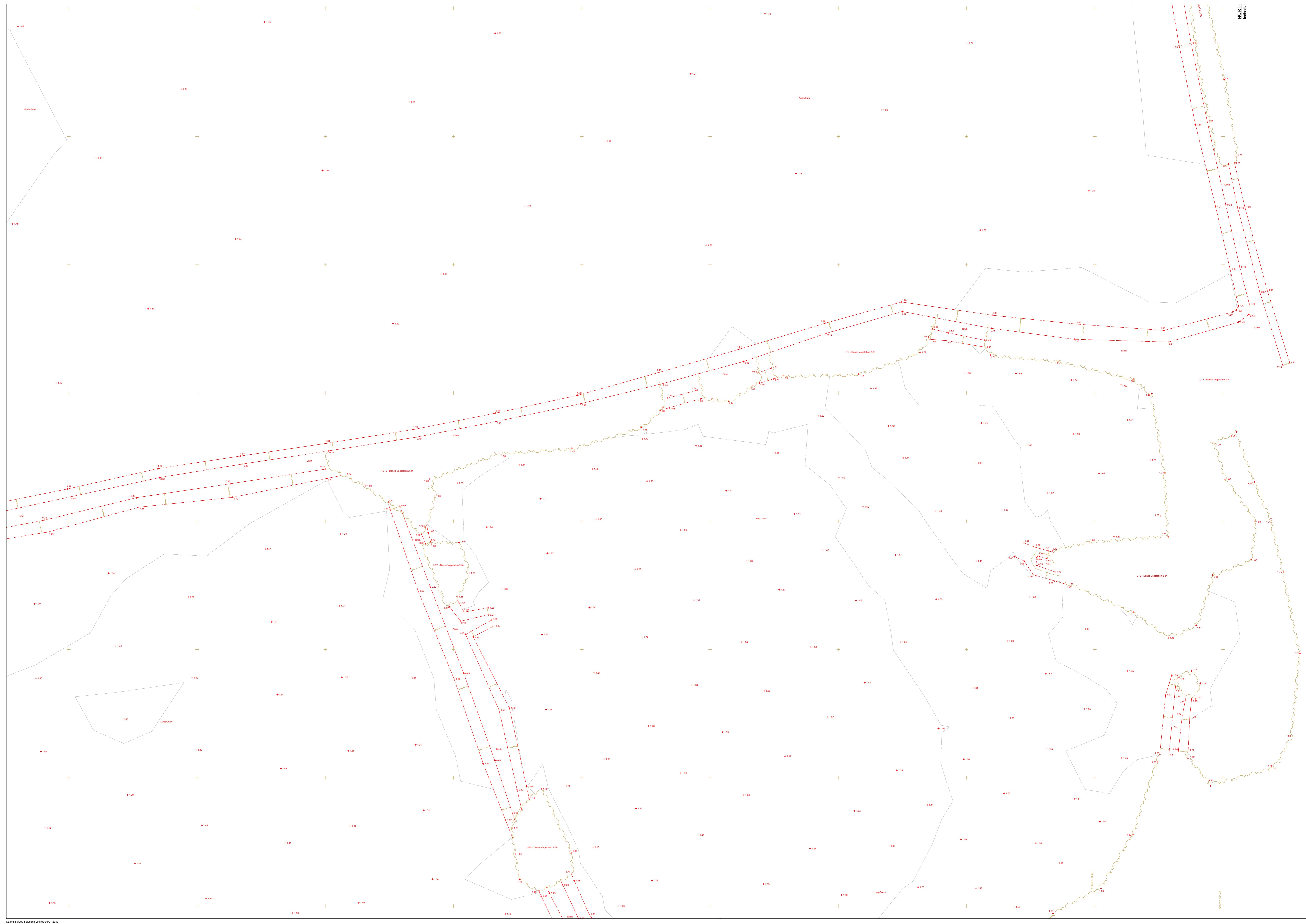
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**LAND SURVEYING BUILDING SURVEYING UNDERGROUND SURVEYING**

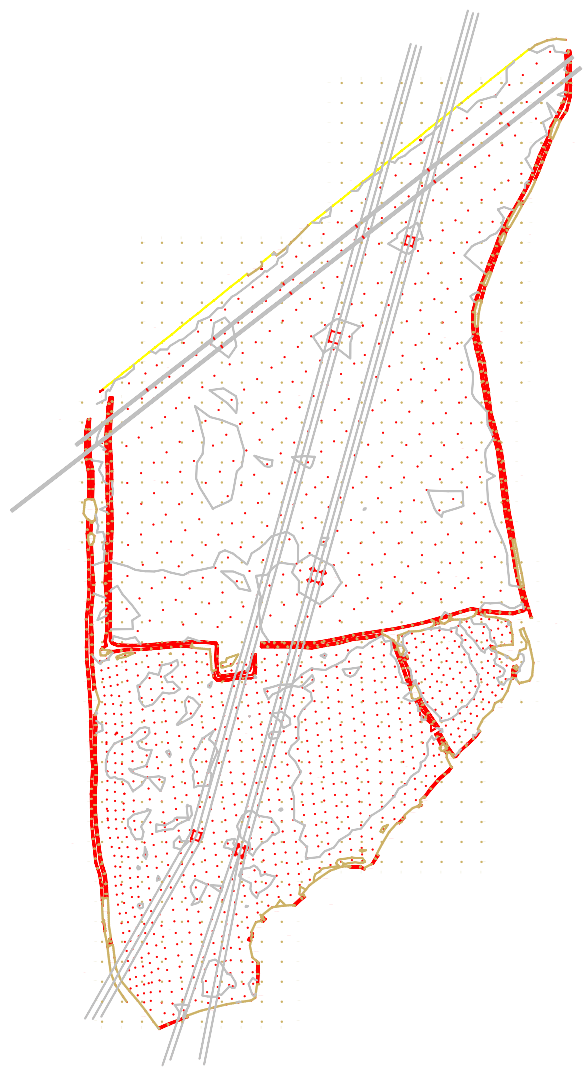
<b>PROJECT TITLE</b> TILBURY POWER STATION, STATION ROAD, TILBURY, ESSEX			
<b>DRAWING DETAIL</b> TOPOGRAPHICAL SURVEY Sheet 7 of 10			
<b>CLIENT</b> SLIGHTLINE LANDSCAPE LTD	<b>SCALE</b> 1:200		
<b>SURVEYOR</b> ADW/BSH	<b>SURVEY DATE</b> 28/02/2018	<b>CHECKED BY</b> JM	<b>APPROVED BY</b> DWS
<b>DRAWING NUMBER</b> 213609a-07	<b>REVISION</b>	<b>ISSUE DATE</b> 13/03/2018	











## Annex 15.1.2: Environment Agency Correspondence

Mr J Morley  
RPS  
[jonathan.morley@rpsgroup.com](mailto:jonathan.morley@rpsgroup.com)

**Our ref** EAn/2018/92748  
**Date** 13 August 2018

Dear Mr Morley

**Enquiry regarding Product 4 for Tilbury**

Thank you for your enquiry which was received on 13 July 2018.

We respond to requests under the Freedom of Information Act 2000 and Environmental Information Regulations 2004.

The information we hold and a copy of the Flood Risk Assessment (FRA) advisory note is attached to my email.

Name	Product 4
Description	Detailed Flood Risk Assessment Map centred on 566300, 176685
Licence	<a href="#">Open Government Licence</a>
Information Warnings	None
Information Warning - OS background mapping	<i>The mapping of features provided as a background in this product is © Ordnance Survey. It is provided to give context to this product. The Open Government Licence does not apply to this background mapping. You are granted a non-exclusive, royalty free, revocable licence solely to view the Licensed Data for non-commercial purposes for the period during which the Environment Agency makes it available. You are not permitted to copy, sub-license, distribute, sell or otherwise make available the Licensed Data to third parties in any form. Third party rights to enforce the terms of this licence shall be reserved to OS.</i>
Attribution	Contains Environment Agency information © Environment Agency and/or database rights.  Contains Ordnance Survey data © Crown copyright 2017 Ordnance Survey 100024198.

**Coastal Modelling**

We are currently undertaking a hydraulic modelling study for the following Essex, Norfolk and Suffolk Coastal areas: Wells, Cromer, Walcott, Thurne, Hickling and Coast, Great Yarmouth, Lowestoft, Kessingland (Lothingland Hundred), Blyth Estuary, Leiston, Alde & Ore Estuary, Deben Estuary, Stour & Orwell Estuary, Clacton, Colne & Blackwater Estuary, Crouch & Roach Estuary, Southend and the Thames.

You may be aware that some Local Planning Authorities have updated their Strategic Flood Risk Assessments (SFRAs) using data from this modelling study. As SFRA's are not updated regularly we agreed that they could use draft outputs as we wanted to ensure that the SFRA's were not out of date as soon as they were published. However although this information was shared with our external partners to assist them with the creation of their SFRAs the data remains unavailable for external practice until model completion. This is because we need to complete all necessary reviews. The project aims to be completed by summer 2018 and will be available for external practice then.

**Additional information**  
**Gifford Gauging station – on the River Mar Dyke.**

Please find attached information from our gauging station. The structure has a theoretical modular limit of 0.79mAC however, non-modular flow can occur from as little as 0.2mAC. The structure is an Essex Standard Weir (Modified flat vee crump) and so its modular limit occurs when the crest level is within 70% of the stage. This structure drowns completely on a frequent basis. There is an operational gate downstream of the structure which does have an effect on the modularity of the site. Additionally, in previous years we have had issues with maintenance and weed growth (2000-2012). Finally, there is a river restoration scheme downstream of the weir which is also believed to have an effect on modularity. This has been in place since ~2015. Flood plain storage begins at 1.1MaC.

**Groundwater**

We have provided a location map showing the location of water abstractions in the area, further details to follow under a separate email in due course.

Information about pollution incidents will follow as soon as possible.

Aquifer vulnerability maps are available on Magic Map using this link: <http://magic.defra.gov.uk/> click search for layers and type in aquifers.

Aquifer designations, soil classifications and Source Protection zones can all be viewed on Magic Map.

Our nearest groundwater observation point monitors the chalk and suggests that ground water levels have varied between 5, and 13.64m below ground level between 1986 and 2008. However, the quality of this data is questionable as it does not seem to fully represent known wet and drought periods.

We only keep records of groundwater flooding events since 2010 and since then we have had no fully verified reports. Groundwater flooding only occurs when the groundwater level exceeds the ground level after periods of high recharge. This typically an uncommon occurrence in this region.

### Data Available Online

Many of our flood datasets are available online:

- Flood Map For Planning ([Flood Zone 2](#), [Flood Zone 3](#), [Flood Storage Areas](#), [Flood Defences](#), [Areas Benefiting from Defences](#))
- [Risk of Flooding from Rivers and Sea](#)
- [Historic Flood Map](#)
- [Current Flood Warnings](#)

### What's In Your BackYard (WIYBY) is no longer available

Most of the data is still available via other sharing services such as [DATA.GOV.UK](#), [MAGIC map](#) and new [GOV.UK digital services](#). Where the datasets are no longer available as maps, you will be able to download and use within specialist applications.

To find out all the services the Environment Agency have available, please click [here](#).

For any other enquiries please send your request to us at:

[Enquiries\\_EastAnglia@environment-agency.gov.uk](mailto:Enquiries_EastAnglia@environment-agency.gov.uk).

### For awareness

Please be aware that we now charge for planning advice provided to developers, agents and landowners. If you would like advice to inform a future planning application for this site then please complete our <https://www.gov.uk/government/publications/pre-planning-application-enquiry-form-preliminary-opinion> and email it to our Sustainable Places team at: [planning.ipswich@environment-agency.gov.uk](mailto:planning.ipswich@environment-agency.gov.uk).

They will initially provide you with a free response identifying the following:

- the environmental constraints affecting the proposal;
- the environmental issues raised by the proposal;
- the information we need for the subsequent planning application to address the issues identified and demonstrate an acceptable development;
- any required environmental permits.

If you require any further information from them (for example, a meeting or the detailed review of a technical document) they will need to set up a charging agreement. Further information can be found on our [website](#).

Please note we have published revised climate change allowances, which are available online. These new allowances will need to be reflected in your Flood Risk Assessment. If you want to discuss this please call our Sustainable Places team on 0203 025 5475.

### East Anglia Area

Ipswich Office, Icen House, Cobham Road, Ipswich, Suffolk, IP3 9JD  
Brampton Office, Bromholme Lane, Brampton, Huntingdon, PE28 4NE  
General Enquiries: 03708 506506  
Email: [enquiries@environment-agency.gov.uk](mailto:enquiries@environment-agency.gov.uk)  
Website: <https://www.gov.uk/government/organisations/environment-agency>

TEAM2100: delivering the first 10 years of investment in tidal flood defences for the Thames Estuary 2100 Plan. For more information, visit [the TEAM2100 website](#) or email [team2100@jacobs.com](mailto:team2100@jacobs.com)

Please get in touch if you have any further queries or contact us within two months if you'd like us to review the information we have sent.

Yours sincerely

*Karen Brown*

**Karen Brown**

**Customers and Engagement Officer**

Direct dial: 02030 255472

### East Anglia Area

Ipswich Office, Icen House, Cobham Road, Ipswich, Suffolk, IP3 9JD  
Brampton Office, Bromholme Lane, Brampton, Huntingdon, PE28 4NE  
General Enquiries: 03708 506506  
Email: [enquiries@environment-agency.gov.uk](mailto:enquiries@environment-agency.gov.uk)  
Website: <https://www.gov.uk/government/organisations/environment-agency>

## Use of Environment Agency Information for Flood Risk Assessments

### **Important**

The Environment Agency are keen to work with partners to enable development which is resilient to flooding for its lifetime and provides wider benefits to communities. If you have requested this information to help inform a development proposal, then we recommend engaging with us as early as possible by using the pre-application form available from our website:

<https://www.gov.uk/government/publications/pre-planning-application-enquiry-form-preliminary-opinion>

We recognise the value of early engagement in development planning decisions. This allows complex issues to be discussed, innovative solutions to be developed that both enables new development and protects existing communities. Such engagement can often avoid delays in the planning process following planning application submission, by reaching agreements up-front. We offer a charged pre-application advice service for applicants who wish to discuss a development proposal.

We can also provide a preliminary opinion for free which will identify environmental constraints related to our responsibilities including flooding, waste, land contamination, water quality, biodiversity, navigation, pollution, water resources, foul drainage or Environmental Impact Assessment.

In preparing your planning application submission, you should refer to the Environment Agency's Flood Risk Standing Advice and the Planning Practice Guidance for information about what flood risk assessment is needed for new development in the different Flood Zones. This information can be accessed via:

<https://www.gov.uk/flood-risk-assessment-standing-advice>  
<http://planningguidance.planningportal.gov.uk/>

You should also consult the Strategic Flood Risk Assessment or other relevant materials produced by your local planning authority.

You should note that:

1. Information supplied by the Environment Agency may be used to assist in producing a Flood Risk Assessment (FRA) where one is required, but does not constitute such an assessment on its own.
2. This information covers flood risk from main rivers and the sea, and you will need to consider other potential sources of flooding, such as groundwater or surface water runoff. Information produced by the local planning authority referred to above may assist here.
3. Where a planning application requires an FRA and this is not submitted or is deficient, the Environment Agency may raise an objection.

**Reference:** EAn/2018/92748  
**Site Address:** 566300,176685  
**Date:** 23/07/2018

### **Included:**

- Flood Map
- Historic Flood Outlines Map

### **Model: Thames TE2100 2008**

- Node Map
- Node Levels

### **Important information to note with your Product:**

#### **Flood Risk Assessments (FRAs)**

If you are obtaining this information for use within a Flood Risk Assessment (FRA) required for a planning application, please include our unaltered Product 4 data within an appendix of your FRA.

#### **Flood Zones**

Please see the attached map showing the Flood Zones (outlines) for the area of the site. Our maps show the site is located in fluvial/ tidal Flood Zone 1/2/3. For further information with regards to Flood Zones, please see below:

#### **Table 1: Flood Zones**

These Flood Zones refer to the probability of river and sea flooding, ignoring the presence of defences.

<b>Flood Zone</b>	<b>Definition</b>
<b>Zone 1 Low Probability</b>	Land having a less than 1 in 1,000 annual probability of river or sea flooding. (Shown as 'clear' on the Flood Map – all land outside Zones 2 and 3)
<b>Zone 2 Medium Probability</b>	Land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding; or land having between a 1 in 200 and 1 in 1,000 annual probability of sea flooding. (Land shown in light blue on the Flood Map)
<b>Zone 3a High Probability</b>	Land having a 1 in 100 or greater annual probability of river flooding; or Land having a 1 in 200 or greater annual probability of sea flooding. (Land shown in dark blue on the Flood Map)

Paragraph: 065 Reference ID: 7-065-20140306

#### **Un-Modelled Watercourses**

We have not undertaken any detailed modelling for the nearby;

- Chadwell Cross Sewer,
- Pincocks Trough,
- West Tilbury Branch Sewer,
- West Tilbury Main,
- St Clares Sewer, and
- Gobions Sewer.

These sources of flood risk have not been assessed for the purpose of the flood map. They will need to be investigated further in any FRA.

Normally, in these circumstances, an FRA will need to undertake a modelling exercise in order to derive flood levels and extents, both with and without allowances for climate change, for these watercourses, in order to inform the design for the site. Without this information, the risk to the development from fluvial or tidal flooding associated with these main watercourses is unknown.

### Historic Flood Events

Examinations of our records of historic flooding show that the general area has previously flooded. Please note that these records show flooding to the land and do not necessarily indicate that properties within the historic flood events were flooded internally. It is also possible that the pattern of flooding in this area has changed and that this area would now flood under different circumstances. Please see the attached PDF for flood history information.

### Surface Water

Please be aware that in recent years, there has been an increase in flood damage caused by surface water flooding or drainage systems that have been overwhelmed. We have worked with Lead local Flood Authorities (LLFAs) to develop a map which incorporates the best local and national scale information on surface water flood risk. These maps can be viewed on our website at the following:-  
<https://flood-warning-information.service.gov.uk/long-term-flood-risk/>

### Reservoir Flooding

You can obtain a map which shows the extent of flooding if a reservoir was to fail and release the water that it holds. The map shows the worst case scenario. These maps can be viewed on our website at the following:-  
<https://flood-warning-information.service.gov.uk/long-term-flood-risk/>



## Thames Estuary 2100 (TE2100)

Location	Node Ref	Easting	Northing	2005		2040		2070		2100		2120		2070 Defence Crest Levels	
				1 in 200 (0.5% AEP)	1 in 1000 (0.1% AEP)	1 in 200 (0.5% AEP)	1 in 1000 (0.1% AEP)	1 in 200 (0.5% AEP)	1 in 1000 (0.1% AEP)	1 in 200 (0.5% AEP)	1 in 1000 (0.1% AEP)	1 in 200 (0.5% AEP)	1 in 1000 (0.1% AEP)	Existing Barrier	New Barrier
				Dartford	3.15	554397	178402	5.64	5.97	5.85	6.18	6.00	6.33	6.32	6.65
Dartford Marshes	3.16	555012	177896	5.62	5.95	5.83	6.16	5.98	6.31	6.30	6.63	6.49	6.82	7.60	6.10
Long Reach	3.17	555831	177179	5.61	5.94	5.82	6.15	5.97	6.30	6.29	6.62	6.48	6.81	7.60	8.50
Dartford Tunnel	3.18	557090	176390	5.61	5.94	5.82	6.15	5.96	6.29	6.27	6.60	6.46	6.79	7.50	8.50
Stone Ness	3.19	557090	175703	5.59	5.92	5.80	6.13	5.95	6.28	6.27	6.60	6.45	6.78	7.50	8.50
West Thurrock	3.20	559355	176131	5.57	5.90	5.78	6.11	5.94	6.27	6.25	6.58	6.43	6.76	7.50	8.50
Swanscombe	3.21	560139	177011	5.56	5.89	5.77	6.10	5.91	6.24	6.22	6.55	6.41	6.74	7.50	8.50
Grays	3.22	561470	176679	5.53	5.86	5.74	6.07	5.91	6.24	6.21	6.54	6.40	6.73	7.50	8.00
Tilbury	3.23	562066	175589	5.52	5.85	5.73	6.06	5.89	6.22	6.19	6.52	6.38	6.71	7.50	8.00
Tilbury	3.24	562675	174950	5.50	5.83	5.71	6.04	5.86	6.19	6.16	6.49	6.36	6.69	7.40	8.00
Tilbury Ferry	3.25	564109	174800	5.48	5.81	5.69	6.02	5.84	6.17	6.14	6.47	6.34	6.67	7.40	8.00
Gravesend	3.26	565307	174848	5.45	5.78	5.66	5.99	5.81	6.14	6.11	6.44	6.32	6.65	7.40	8.00
Gravesend Power S	3.27	566916	174908	5.38	5.71	5.59	5.92	5.75	6.08	6.05	6.38	6.28	6.61	7.40	8.00
East Tilbury Mairs	3.28	568488	175258	5.31	5.64	5.52	5.85	5.68	6.01	5.99	6.32	6.23	6.56	7.00	7.00
Coalhouse Point	3.29	569850	176137	5.25	5.58	5.46	5.79	5.60	5.93	5.92	6.25	6.18	6.51	6.48	6.48
Coastguard Cottage	3.30	570320	176011	5.21	5.54	5.42	5.75	5.56	5.89	5.86	6.19	6.13	6.46	6.75	6.75
Mucking Flats	3.31	571235	179824	5.16	5.49	5.37	5.70	5.53	5.86	5.85	6.18	6.12	6.45	7.50	8.10
Corringham Marshes	3.32	573440	180782	5.08	5.41	5.29	5.62	5.48	5.81	5.83	6.16	6.10	6.43	7.50	8.10
Blythe Sands	3.33	575633	181137	5.00	5.33	5.21	5.54	5.43	5.76	5.81	6.14	6.08	6.41	7.50	8.10
Halstow Marshes	3.34	577953	181149	4.95	5.28	5.16	5.49	5.37	5.70	5.76	6.09	6.04	6.37	7.40	8.10
West Point	3.35	579995	181222	4.89	5.22	5.10	5.43	5.33	5.66	5.72	6.05	6.01	6.34	7.40	8.10
East Canvey Point	3.36	583007	181318	4.81	5.14	5.02	5.35	5.30	5.63	5.69	6.02	5.98	6.31	7.40	8.10
Leigh	3.37	585820	181583	4.73	5.06	4.94	5.27	5.27	5.60	5.66	5.99	5.95	6.28	6.70	7.40
Southend	3.38	588653	181517	4.70	5.03	4.91	5.24	5.22	5.55	5.62	5.95	5.92	6.25	6.70	7.40

### Thames Estuary 2100 (TE2100)

You have requested in-channel flood levels for the tidal river Thames. These have been taken from the Thames Estuary 2100 study completed by HR Wallingford in 2008.

### Details about the TE2100 plan

The TE2100 plan is now live and within it are a set of levels on which the flood risk management strategy is based. The plan is the overarching flood management strategy for the Thames Estuary and therefore any development planning should be based on the same underlying data.

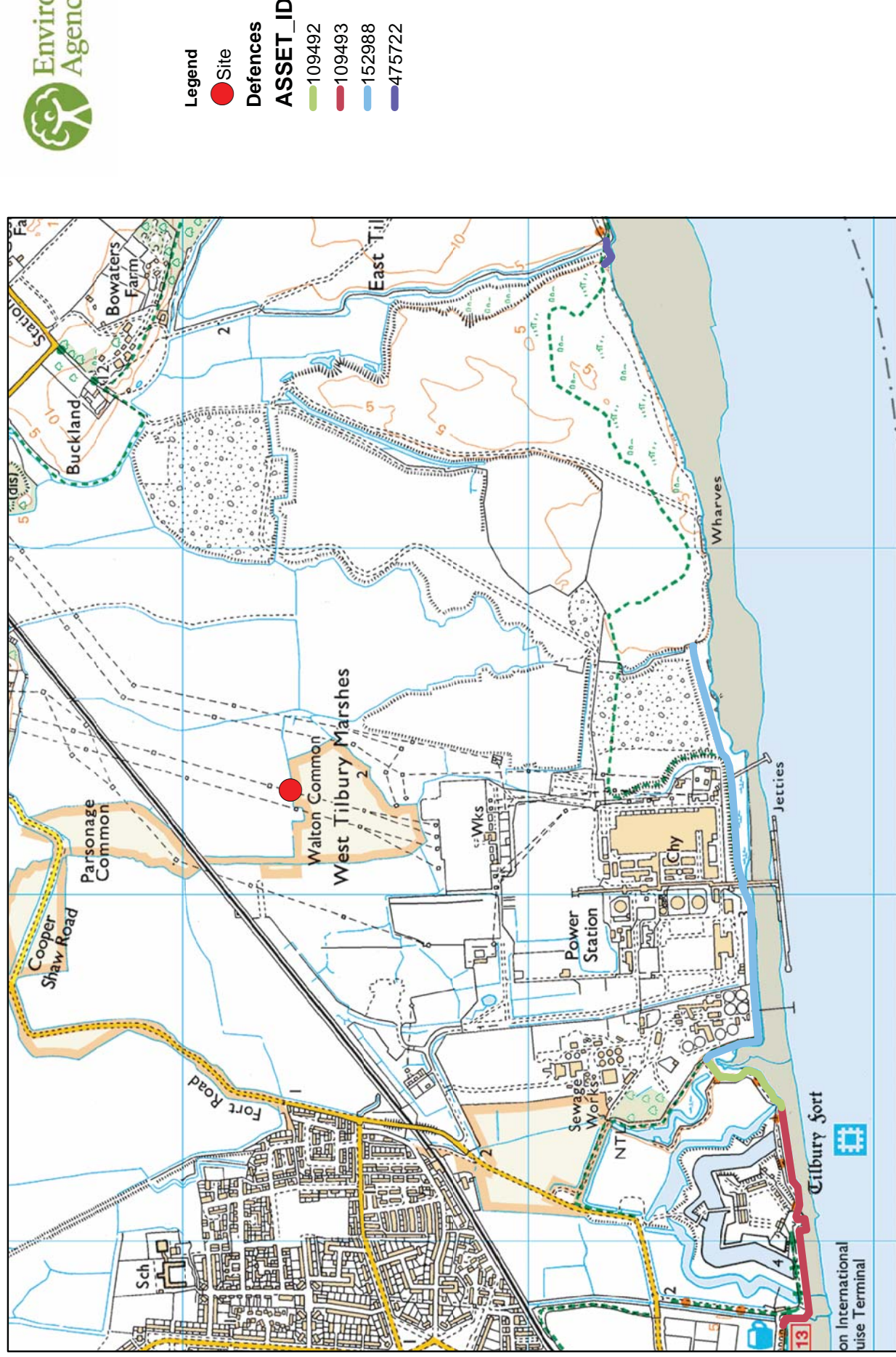
### Details about the TE2100 in-channel levels

The TE2100 in-channel levels take into account operation of the Thames Barrier when considering future levels. The Thames Barrier requires regular maintenance and with additional closures the opportunity for maintenance will be reduced. When this happens, river levels – for which the Barrier would normally shut for the 2008 epoch – will have to be allowed through to ensure that the barrier is not shut too often. For this reason, levels upriver of the barrier will increase and the tidal walls will need to be heightened to match.



# Flood Defence Location Map centred on West Tilbury Marshes

## Ref: EAN/2018/92748



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Scale 1:15,000

Contact Us: National Customer Contact Centre, PO Box 544, Rotherham, S60 1BY. Tel: 03708 506 506 (Mon-Fri 8-6). Email: enquiries@environment-agency.gov.uk

Date: 18/07/18  
Datasheet Reference: EAN/2018/92748

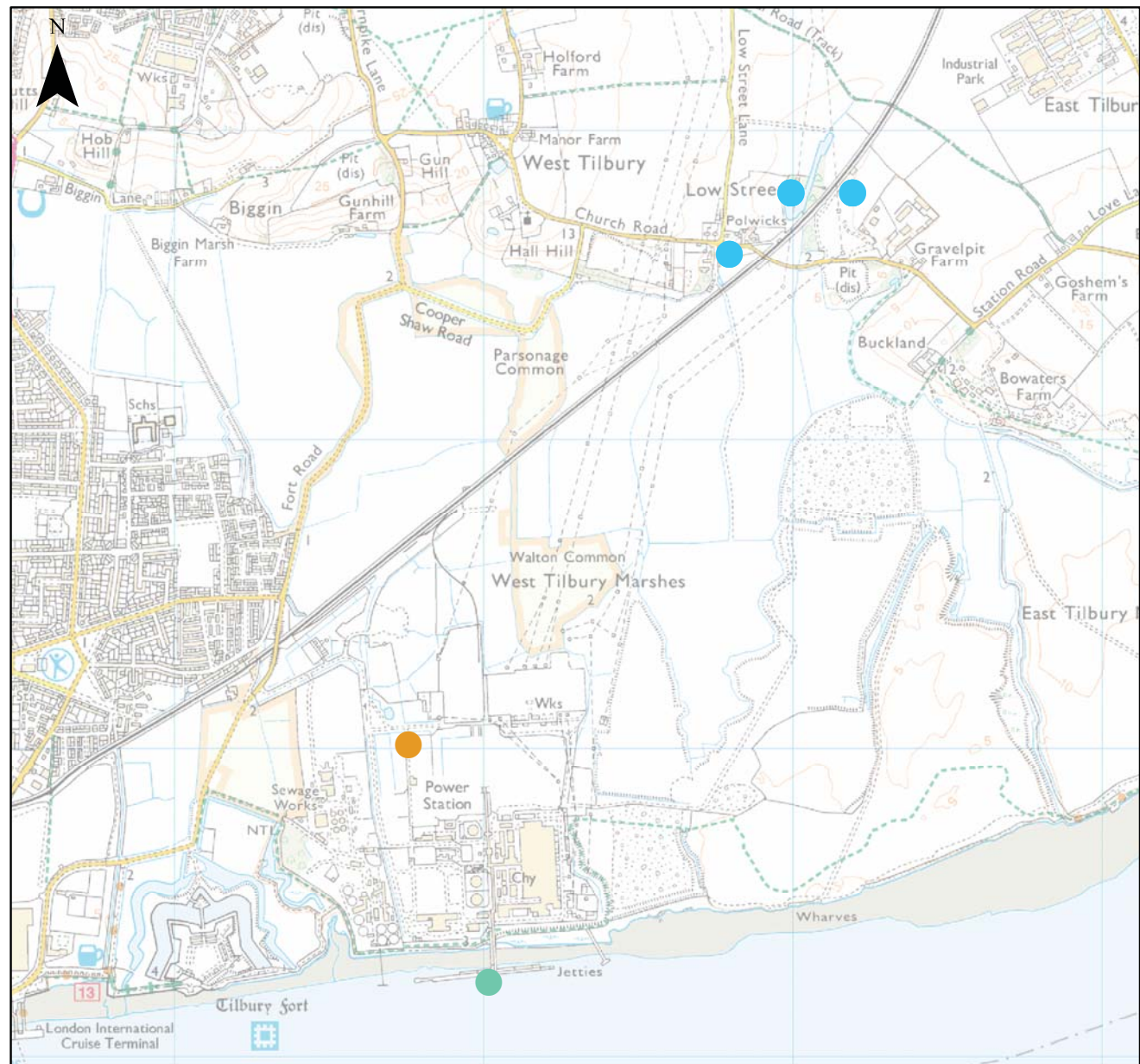


### Defence Information

Asset Reference	Maintainer	Bank	Asset Type	Asset Description	Standard of Protection	Overall Condition Grade	Crest Level
109492	Environment Agency	left	wall	Sheet Piling Wall	1000.0	3	6.700
109493	Environment Agency	left	wall	Piling with embankment	1000.0	4	6.670
152988	Environment Agency	left	wall	Wall	1000.0	5	6.480
475722	Environment Agency		wall	Concrete Wall	1000.0	3	6.000

### Key to Overall Condition Grades

Grade	Rating	Description
1	Very Good	Cosmetic Defects that will have no effect on performance.
2	Good	Minor defects that will not reduce the overall performance of the asset.
3	Fair	Defects that could reduce performance of the asset
4	Poor	Defects that would significantly reduce the performance of the asset. Further investigation.
5	Very Poor	Severe defects resulting in complete performance failure.



**Abstraction Licence Type**

- Deregulated
- Groundwater
- Tidal

## Annex 15.1.3: MicroDrainage Outputs

ICP SUDS Mean Annual Flood

Input

Return Period (years)	2	Soil	0.400
Area (ha)	17.000	Urban	0.000
SAAR (mm)	545	Region Number	Region 6

**Results 1/s**

QBAR Rural 43.2  
QBAR Urban 43.2

Q2 years 38.0

Q1 year 36.7  
Q30 years 97.8  
Q100 years 137.7

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Overflow (l/s)	Max Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	8.794	0.794	22.5	0.0	22.5	8338.5	O K
30 min Summer	8.875	0.875	23.7	0.0	23.7	9189.8	O K
60 min Summer	8.964	0.964	24.9	0.0	24.9	10116.9	O K
120 min Summer	9.058	1.058	26.2	0.0	26.2	11113.5	O K
180 min Summer	9.116	1.116	27.0	0.0	27.0	11720.1	O K
240 min Summer	9.158	1.158	27.5	0.0	27.5	12155.5	O K
360 min Summer	9.216	1.216	28.2	0.0	28.2	12765.1	O K
480 min Summer	9.256	1.256	28.7	0.0	28.7	13184.8	O K
600 min Summer	9.285	1.285	29.0	0.0	29.0	13495.4	O K
720 min Summer	9.308	1.308	29.3	0.0	29.3	13734.7	O K
960 min Summer	9.403	1.403	30.4	0.0	30.4	14730.8	O K
1440 min Summer	9.536	1.536	31.9	0.0	31.9	16131.8	O K
2160 min Summer	9.661	1.661	33.2	0.0	33.2	17441.0	O K
2880 min Summer	9.737	1.737	33.9	0.0	33.9	18234.4	Flood Risk
4320 min Summer	9.674	1.674	33.3	0.0	33.3	17572.6	O K
5760 min Summer	9.620	1.620	32.7	0.0	32.7	17012.7	O K
7200 min Summer	9.573	1.573	32.2	0.0	32.2	16516.0	O K
8640 min Summer	9.529	1.529	31.8	0.0	31.8	16054.4	O K
10080 min Summer	9.487	1.487	31.3	0.0	31.3	15610.6	O K
15 min Winter	8.890	0.890	23.9	0.0	23.9	9342.7	O K
30 min Winter	8.981	0.981	25.2	0.0	25.2	10297.1	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Overflow Volume (m³)	Time-Peak (mins)
15 min Summer	263.696	0.0	1790.8	0.0	83
30 min Summer	145.425	0.0	1884.7	0.0	97
60 min Summer	80.200	0.0	3784.4	0.0	126
120 min Summer	44.229	0.0	3976.7	0.0	184
180 min Summer	31.226	0.0	4078.8	0.0	242
240 min Summer	24.392	0.0	4142.4	0.0	302
360 min Summer	17.221	0.0	4211.9	0.0	418
480 min Summer	13.452	0.0	4239.7	0.0	536
600 min Summer	11.106	0.0	4243.4	0.0	652
720 min Summer	9.497	0.0	4231.3	0.0	770
960 min Summer	7.754	0.0	4283.7	0.0	1004
1440 min Summer	5.827	0.0	4242.2	0.0	1476
2160 min Summer	4.379	0.0	8995.2	0.0	2184
2880 min Summer	3.576	0.0	8963.2	0.0	2892
4320 min Summer	2.499	0.0	8131.5	0.0	3872
5760 min Summer	1.938	0.0	16346.7	0.0	4560
7200 min Summer	1.591	0.0	15797.2	0.0	5280
8640 min Summer	1.355	0.0	15102.3	0.0	6080
10080 min Summer	1.182	0.0	14280.7	0.0	6888
15 min Winter	263.696	0.0	1910.6	0.0	82
30 min Winter	145.425	0.0	2009.4	0.0	97

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Overflow (l/s)	Max Outflow (l/s)	Max Volume (m³)	Status
60 min Winter	9.080	1.080	26.5	0.0	26.5	11337.4	O K
120 min Winter	9.187	1.187	27.8	0.0	27.8	12458.4	O K
180 min Winter	9.252	1.252	28.6	0.0	28.6	13142.4	O K
240 min Winter	9.299	1.299	29.2	0.0	29.2	13634.4	O K
360 min Winter	9.364	1.364	29.9	0.0	29.9	14326.7	O K
480 min Winter	9.410	1.410	30.5	0.0	30.5	14806.4	O K
600 min Winter	9.444	1.444	30.9	0.0	30.9	15164.1	O K
720 min Winter	9.471	1.471	31.1	0.0	31.1	15441.9	O K
960 min Winter	9.579	1.579	32.3	0.0	32.3	16579.9	O K
1440 min Winter	9.733	1.733	33.9	0.0	33.9	18195.7	Flood Risk
2160 min Winter	9.880	1.880	35.4	0.0	35.4	19734.9	Flood Risk
<b>2880 min Winter</b>	<b>9.972</b>	<b>1.972</b>	<b>36.2</b>	<b>0.0</b>	<b>36.2</b>	<b>20703.2</b>	<b>Flood Risk</b>
4320 min Winter	9.911	1.911	35.7	0.0	35.7	20067.9	Flood Risk
5760 min Winter	9.836	1.836	34.9	0.0	34.9	19276.2	Flood Risk
7200 min Winter	9.778	1.778	34.4	0.0	34.4	18670.3	Flood Risk
8640 min Winter	9.721	1.721	33.8	0.0	33.8	18071.7	Flood Risk
10080 min Winter	9.665	1.665	33.2	0.0	33.2	17479.2	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Overflow Volume (m³)	Time-Peak (mins)
60 min Winter	80.200	0.0	4047.7	0.0	126
120 min Winter	44.229	0.0	4250.5	0.0	182
180 min Winter	31.226	0.0	4358.1	0.0	240
240 min Winter	24.392	0.0	4425.3	0.0	296
360 min Winter	17.221	0.0	4498.7	0.0	412
480 min Winter	13.452	0.0	4528.0	0.0	526
600 min Winter	11.106	0.0	4532.1	0.0	642
720 min Winter	9.497	0.0	4519.4	0.0	756
960 min Winter	7.754	0.0	4575.0	0.0	986
1440 min Winter	5.827	0.0	4532.1	0.0	1448
2160 min Winter	4.379	0.0	9638.5	0.0	2140
<b>2880 min Winter</b>	<b>3.576</b>	<b>0.0</b>	<b>9601.3</b>	<b>0.0</b>	<b>2820</b>
4320 min Winter	2.499	0.0	8719.3	0.0	4116
5760 min Winter	1.938	0.0	17659.4	0.0	4768
7200 min Winter	1.591	0.0	17055.1	0.0	5592
8640 min Winter	1.355	0.0	16302.0	0.0	6512
10080 min Winter	1.182	0.0	15419.6	0.0	7416


Rainfall Details

Rainfall Model	FEH	D2 (1km) 0.415	Cv (Summer) 0.750
Return Period (years)	100	D3 (1km) 0.236	Cv (Winter) 0.840
FEH Rainfall Version	1999	E (1km) 0.320	Shortest Storm (mins) 15
Site Location		F (1km) 2.576	Longest Storm (mins) 10080
	C (1km) -0.026	Summer Storms Yes	Climate Change % +40
	D1 (1km) 0.261	Winter Storms Yes	

Time Area Diagram

Total Area (ha) 17.000

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From: 0	To: 4	From: 20	To: 24	From: 40	To: 44	From: 60	To: 64
1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
4	8	24	28	44	48	64	68
1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
8	12	28	32	48	52		
1.000	1.000	1.000	1.000	1.000	1.000		
12	16	32	36	52	56		
1.000	1.000	1.000	1.000	1.000	1.000		
16	20	36	40	56	60		
1.000	1.000	1.000	1.000	1.000	1.000		

RPS Group PLC		Page 4
Suite D10 Josephs Well Leeds LS3 1AB		
Date 08/10/2018 15:19 File THURROCK FGP - TANK 1IN...	Designed by jonathan.morley Checked by	
Micro Drainage	Source Control 2017.1.2	

Model Details

Storage is Online Cover Level (m) 10.000

Tank or Pond Structure

Invert Level (m) 8.000

Depth (m)	Area (m <sup>2</sup> )	Depth (m)	Area (m <sup>2</sup> )	Depth (m)	Area (m <sup>2</sup> )
0.000	10500.0	0.800	10500.0	1.600	10500.0
0.400	10500.0	1.200	10500.0	2.000	10500.0

Orifice Outflow Control

Diameter (m) 0.112 Discharge Coefficient 0.600 Invert Level (m) 8.000

Weir Overflow Control

Discharge Coef 0.544 Width (m) 1.000 Invert Level (m) 10.000